GENERAL CONDITIONS AND
TECHNICAL SPECIFICATIONS
for
Construction of
SEWER LINE ADDITIONS
for the
TOWN OF SMYRNA
In
RUTHERFORD COUNTY, TENNESSEE

Updated January 13, 2016
Good Morning Mark,

Thank you for your letter dated January 14, 2019, requesting a one year extension for project WPN 16-0048 [Specifications for Construction of Sewer Line Additions]. This approval expires on February 8, 2019.

Your request is granted, therefore, the approval for WPN 16-0048 expires on February 8, 2020.

Please note that I have placed a comment in our database (WaterLog) for confirmation and future reference of this one year extension (see excerpt below).

Hope this helps—take care,

Robert G. O'Dette, M.S., P.E. BCEE| WEF FELLOW
Division of Water Resources
Tennessee Department of Environment and Conservation
William R. Snodgrass Tennessee Tower, 11th Floor
312 Rosa L. Parks Avenue
Nashville, TN 37243-1102
Office: (615) 532-0625
Direct: (615) 253-5319
Fax: (615) 532-0686
February 8, 2016

Mr. Mark Parker, P.E.
Assistant Director of Utilities
Town of Smyrna
e-copy: mark.parker@townofsmyrna.org
315 S. Lowery Street
Smyrna, TN 37167

Subject: Smyrna Utilities
County: Rutherford
Wastewater Project Number: 16-0048
Project: General Conditions & Technical Specifications for Sewer Line Additions

Dear Mr. Parker:

The Tennessee Department of Environment and Conservation, Division of Water Resources, acknowledges the receipt of 5 sets of documents on January 15, 2016.

Review of these standard sanitary sewer specifications shows that they are in conformance with our guidelines. Therefore, they have been stamped "APPROVED". This approval will remain in effect until February 8, 2019.

We are returning 4 copies to you and retaining one copy for our files. To expedite matters, please reference the assigned wastewater project number 16-0048 on any future correspondence. If we may be of any assistance, please feel free to contact Mr. Adnan Bahour, Ph.D. at (615) 532-0638 or by E-mail at Adnan.Bahour@tn.gov.

Sincerely,

Vojin Janjić
Manager, Water-Based Systems

cc: Water-Based Systems File
    Ms. Ann M. Morbit, Unit Manager, TDEC Division of Water Resources, Ann.Morbitt@tn.gov
TOWN OF SMYRNA

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GC-1 DEFINITIONS

Wherever used in the CONTRACT DOCUMENTS, the following terms shall have the meanings indicated which shall be applicable to both the singular and plural thereof:

GC-1.02 ADDENDA - Written or graphic instruments issued prior to the execution of the Agreement, which modify or interpret the CONTRACT DOCUMENTS, DRAWINGS and SPECIFICATIONS, by additions, deletions, clarifications or corrections.

GC-1.03 BID - The offer or proposal of the BIDDER submitted on the prescribed form setting forth the prices for the WORK to be performed.

GC-1.04 BIDDER - Any person, firm or corporation submitting a BID for the WORK.

GC-1.05 BONDS - Bid, Performance, and Payment Bonds and other instruments of security, furnished by the CONTRACTOR and his security in accordance with the CONTRACT DOCUMENTS.

GC-1.06 CHANGE ORDER - A written order to the CONTRACTOR authorizing an addition, deletion or revision in the WORK within the general scope of the CONTRACT DOCUMENTS, or authorizing an adjustment in the CONTRACT PRICE or CONTRACT TIME.

GC-1.07 CONTRACT DOCUMENTS - The contract, including Advertisement for Bids, Information for Bidders, BID, Bid Bond, Agreement, Payment Bond, Performance Bond, NOTICE TO AWARD, NOTICE TO PROCEED, CHANGE ORDER, DRAWINGS, SPECIFICATIONS, and ADDENDA.

GC-1.08 CONTRACT PRICE - The total monies payable to the CONTRACTOR under the terms and conditions of the CONTRACT DOCUMENTS.

GC-1.09 CONTRACT TIME - The number of calendar days stated in the CONTRACT DOCUMENTS for the completion of the WORK.

GC-1.10 CONTRACTOR - The person, firm or corporation with whom the OWNER has executed the Agreement.

GC-1.11 DRAWINGS - The part of the CONTRACT DOCUMENTS, which show the characteristics and scope of the WORK to be performed and which have been prepared or approved by the ENGINEER.

GC-1.12 ENGINEER - The person, firm or corporation named as such in the CONTRACT DOCUMENTS.

GC-1.13 FIELD ORDER - A written order effecting a change in the WORK not involving an adjustment in the CONTRACT PRICE or an extension of the CONTRACT TIME,
issued by the ENGINEER to the CONTRACTOR during construction.

**NOTICE OF AWARD** – The written notice of the acceptance of the BID from the OWNER to the successful BIDDER.

**NOTICE TO PROCEED** – Written communication issued by the OWNER to the CONTRACTOR authorizing him to proceed with the WORK and establishing the date of commencement of the WORK.

**OWNER** – a public or quasi-public body or authority, corporation, association, partnership, or individual for whom the WORK is to be performed.

**PROJECT** – The undertaking to be performed as provided in the CONTRACT DOCUMENTS.

**RESIDENT PROJECT REPRESENTATIVE** – The authorized representative of the OWNER who is assigned to the PROJECT site or any part thereof.

**SHOP DRAWINGS** – All drawings, diagrams, illustrations, brochures, schedules, and other data which are prepared by the CONTRACTOR, a SUBCONTRACTOR, manufacturer, SUPPLIER or distributor, which illustrate how specific portions of the WORK shall be fabricated or installed.

**SPECIFICATIONS** – A part of the CONTRACT DOCUMENTS consisting of written descriptions of a technical nature of materials, equipment, construction systems, standards and workmanship.

**SUBCONTRACTOR** – An individual, firm or corporation having a direct contract with the CONTRACTOR or with any other SUBCONTRACTOR for the performance of a part of the WORK at the site.

**SUBSTANTIAL COMPLETION** – That date as certified by the ENGINEER when the construction of the PROJECT or a specified part thereof is sufficiently completed, in accordance with the CONTRACT DOCUMENTS, so that the PROJECT or specified part can be utilized for the purposes for which it is intended.

**SUPPLEMENTAL GENERAL CONDITIONS** – Modifications to General Conditions required by a Federal agency for participation in the PROJECT and approved by the agency in writing prior to inclusion in the CONTRACT DOCUMENTS.

**SUPPLIERS** – Any person, supplier or organization who supplies materials or equipment for the WORK, including that fabricated to a special design, but who does not perform labor at the site.

**WORK** – All labor necessary to produce the construction required by the CONTRACT DOCUMENTS, and all materials and equipment incorporated or to be incorporated in the PROJECT.
GC-1.26 **WRITTEN NOTICE** – Any notice to any party of the Agreement relative to any part of this Agreement in writing and considered delivered and the service thereof completed, when posted by certified or registered mail to the said party at his last given address, or delivered in person to said party or his authorized representative on the **WORK**.

**ADDITIONAL INSTRUCTIONS AND DETAIL DRAWINGS**

**GC-2.01 Thru 2.02**

**GC-2**

**ADDITIONAL INSTRUCTIONS AND DETAIL DRAWINGS**

**GC-2.01**

The **CONTRACTOR** may be furnished additional instructions and detail drawings, by the **ENGINEER**, as necessary to carry out the **WORK** required by the **CONTRACT DOCUMENTS**.

**GC-2.02**

The additional drawings and instruction thus supplied will become a part of the **CONTRACT DOCUMENTS**. The **CONTRACTOR** shall carry out the **WORK** in accordance with the additional detail drawings and instructions.

**SCHEDULES, REPORTS AND RECORDS**

**GC-3.01 Thru 3.03**

**GC-3**

**SCHEDULES, REPORTS AND RECORDS**

**GC-3.01**

The **CONTRACTOR** shall submit to the **OWNER** such schedule of quantities and costs, progress schedules, payrolls, reports, estimates, records and data as the **OWNER** may request concerning **WORK** performed or to be performed.

**GC-3.02**

Prior to the first partial payment estimate the **CONTRACTOR** shall submit schedules showing the order in which he proposes to carry on the **WORK**, including dates at which he will start the various parts of the **WORK**, estimated date of completion of each part and, as applicable:

**GC-3.2.1** the dates at which special detail drawings will be required; and

**GC-3.2.2** respective dates for submission of **SHOP DRAWINGS**, the beginning of manufacture, the testing and the installation of materials, supplies and equipment.

**GC-3.03**

The **CONTRACTOR** shall also submit a schedule of payments that he anticipates he will earn during the course or the **WORK**.
The intent of the DRAWINGS and SPECIFICATIONS is that the CONTRACTOR shall furnish all labor, materials, tools, equipment, and transportation necessary for the proper execution of the WORK in accordance with the CONTRACT DOCUMENTS and all incidental work necessary to complete the PROJECT in an acceptable manner, ready for use, occupancy or operation by the OWNER.

In case of conflict between the DRAWINGS and SPECIFICATIONS the SPECIFICATIONS shall govern. Figure dimensions on DRAWINGS shall govern over scale dimensions, and detailed DRAWINGS shall govern over general DRAWINGS.

Any discrepancies found between the DRAWINGS and SPECIFICATIONS and site conditions or any inconsistencies or ambiguities in the DRAWINGS and SPECIFICATIONS shall be immediately reported to the ENGINEER, in writing, who shall promptly correct such inconsistencies or ambiguities in writing. WORK done by the CONTRACTOR after his discovery of such discrepancies, inconsistencies or ambiguities shall be done at the CONTRACTOR'S risk.

The CONTRACTOR shall provide SHOP DRAWINGS as may be necessary for the prosecution of the WORK as required by the CONTRACT DOCUMENTS. The ENGINEER shall promptly review all SHOP DRAWINGS. The ENGINEER'S approval of any SHOP DRAWING shall not release the CONTRACTOR from responsibility for deviations from the CONTRACT DOCUMENTS. The approval of any SHOP DRAWINGS, which substantially deviates from the requirement of the CONTRACT DOCUMENTS, shall be evidenced by a CHANGE ORDER.

When submitted for the ENGINEER'S review, SHOP DRAWINGS shall bear the CONTRACTOR'S certification that he has reviewed, checked and approved the SHOP DRAWINGS and that they are in conformance with the requirements of the CONTRACT DOCUMENTS.

 Portions of the WORK requiring a SHOP DRAWING or sample submission shall not begin until the SHOP DRAWING or submission has been approved by the ENGINEER. A copy of each approved SHOP DRAWING and each approved sample shall be kept in good order by the CONTRACTOR at the site and shall be available to the ENGINEER.
MATERIALS, SERVICES, AND FACILITIES  GC-6.01 Thru 6.05

GC-6  MATERIALS, SERVICES, AND FACILITIES

GC-6.01  It is understood that, except as otherwise specifically stated in the CONTRACT DOCUMENTS, the CONTRACTOR shall provide and pay for the materials, labor, tools, equipment, water, light, power, transportation, supervision, temporary construction of any nature, and all other services and facilities of any nature whatsoever necessary to execute, complete, and deliver the WORK within the specified time.

GC-6.02  Materials and equipment shall be so stored as to insure the preservation of their quality and fitness for the WORK. Stored materials and equipment to be incorporated in the WORK shall be located so as to facilitate prompt inspection.

GC-6.03  Manufactured articles, materials and equipment shall be applied, installed, connected, erected, used, cleaned and conditioned as directed by the manufacturer.

GC-6.04  Materials, supplies and equipment shall be in accordance with samples submitted by the CONTRACTOR and approved by the ENGINEER.

GC-6.05  Materials, supplies or equipment to be incorporated into the WORK shall not be purchased by the CONTRACTOR or the SUBCONTRACTOR subject to a chattel mortgage or under a conditional sale contract or other agreement by which an interest is retained by the seller.

INSPECTIONS AND TESTING  GC-7.01 Thru 7.07

GC-7  INSPECTIONS AND TESTING

GC-7.01  All materials and equipment used in the construction of the PROJECT shall be subject to adequate inspection and testing in accordance with generally accepted standards.

GC-7.02  The CONTRACTOR shall provide at his expense the necessary testing and inspection services required by the CONTRACT DOCUMENTS, unless otherwise provided.

GC-7.03  The OWNER shall provide all other inspection and testing services not required by the CONTRACT DOCUMENTS.

GC-7.04  If the CONTRACT DOCUMENTS, laws, ordinances, rules, regulations or orders of any public authority having jurisdiction require any WORK to specifically be inspected, tested, or approved by someone other than the CONTRACTOR, the CONTRACTOR will give the ENGINEER timely notice of readiness. The CONTRACTOR will then furnish the ENGINEER the required certificates of inspection, testing or approval.

GC-7.05  Neither observations by the ENGINEER nor inspections, tests or approvals by
persons other than the CONTRACTOR shall relieve the CONTRACTOR from his obligations to perform the WORK in accordance with the requirements of the CONTRACT DOCUMENTS.

GC-7.06 The ENGINEER and his representatives will at all times have access to the WORK. In addition, authorized representatives and agents of any participating Federal or State agency shall be permitted to inspect the work, materials, payrolls, records of personnel, invoices of materials, and other relevant data and records. The CONTRACTOR will provide proper facilities for such access and observation of the WORK and also for any inspection, or testing thereof.

GC-7.07 If any WORK is covered contrary to the written request of the ENGINEER it must, if requested by the ENGINEER, be uncovered for his observation and replaced at the CONTRACTOR's expense.

GC-7.08 If the ENGINEER considers it necessary or advisable that covered WORK be inspected or tested by others, the CONTRACTOR, at the ENGINEER'S request, will uncover, expose or otherwise make available for observation, inspection or testing as the ENGINEER may require, that portion of the WORK in question, furnishing all necessary labor, materials, tools, and equipment. If it is found that such WORK is defective, the CONTRACTOR will bear all the expenses of such uncovering, exposure, observation, inspection and testing and of satisfactory reconstruction. If, however, such WORK is not found to be defective, the CONTRACTOR will e allowed an increase in the CONTRACT PRICE or an extension of the CONTRACT TIME, or both, directly attributable to such uncovering, exposure, observation, inspection, testing and reconstruction and an appropriate CHANGE ORDER shall be issued.

SUBSTITUTIONS GC-8.01 Thru 8.01

GC-8 SUBSTITUTIONS

GC-8.01 Whenever a material, article or piece of equipment is identified on the DRAWINGS or SPECIFICATIONS by reference to brand name or catalogue number, it shall be understood that this is referenced for the purpose of defining the performance or other salient requirements and that other products of equal capacities, quality and function shall be considered. The CONTRACTOR may recommend the substitution of a material, article, or piece of equipment of equal substance and function for those referred to in the CONTRACT DOCUMENTS by reference to brand name or catalogue number, and if, in the opinion of the ENGINEER, such material, article, or piece of equipment is of equal substance and function to that specified, the ENGINEER may approve its substitution and use by the CONTRACTOR. Any cost differential shall be deductible from the CONTRACT

11
PRICE and the CONTRACT DOCUMENTS shall be appropriately modified by
CHANGE ORDER.

The CONTRACTOR warrants that if substitutes are approved, no major changes in
the function or general design of the PROJECT will result. Incidental changes or
extra component parts required to accommodate the substitute will be made by the
CONTRACTOR without a change in the CONTRACT PRICE or CONTRACT TIME.

PATENTS

PATENTS

GC-9.01 Thru 9.01

The CONTRACTOR shall pay all applicable royalties and license fees. He shall
defend all suits or claims for infringement of any patent rights and save OWNER
harmless from loss on account thereof, except that the OWNER shall be
responsible for any such loss when a particular process, design, or the product of a
particular manufacturer or manufacturers is specified, but if the CONTRACTOR
has reason to believe that the design, process or product specified is an
infringement of a patent, he shall be responsible for such loss unless he promptly
gives such information to the ENGINEER.

SURVEYS, PERMITS, REGULATIONS

GC-10.01 Thru 10.03

The OWNER shall furnish all boundary surveys and establish all base lines for
locating the principal component parts of the WORK together with a suitable
number of benchmarks adjacent to the WORK as shown in the CONTRACT
DOCUMENTS. From the information provided by the OWNER, unless otherwise
specified in the CONTRACT DOCUMENTS, the CONTRACTOR shall develop and
make all detail surveys needed for construction such as slope stakes, batter
boards, stakes for pile locations and other working points, lines, elevations and cut
sheets.

The CONTRACTOR shall carefully preserve bench marks, reference points and
stakes and, in case of willful or careless destruction, he shall be charged with the
resulting expense and shall be responsible for any mistakes that may be caused by
their unnecessary loss or disturbance.

Permits and licenses of a temporary nature necessary for the prosecution of the
WORK shall be secured and paid for by the CONTRACTOR. Permits, licenses and
easements for permanent structures or permanent changes in existing facilities
shall be secured and paid for by the OWNER, unless otherwise specified. The
The CONTRACTOR shall give all notices and comply with all laws, ordinances, rules
and regulations bearing on the conduct of the WORK as drawn and specified. If the
CONTRACTOR observes that the CONTRACT DOCUMENTS are at variance
therewith, he shall promptly notify the ENGINEER in writing, and any necessary
changes shall be adjusted as provided in Section 13, CHANGES IN THE WORK.

PROTECTION OF WORK, PROPERTY AND PERSONS GC-11.01 Thru 11.03

GC-11 PROTECTION OF WORK, PROPERTY AND PERSONS

GC-11.01 The CONTRACTOR will be responsible for initiating, maintaining and supervising all
safety precautions and programs in connection with the WORK. He will take all
necessary precautions for the safety of, and will provide the necessary protection to
prevent damage, injury or loss to all employees on the WORK and other persons
who may be affected thereby, all the WORK and all materials or equipment to be
incorporated therein, whether in storage on or off the site, and other property at the
site or adjacent thereto, including trees, shrubs, lawns, walks, pavements,
roadways, structures and utilities not designated for removal, relocation or
replacement in the courses of construction.

GC-11.02 The CONTRACTOR will comply with all applicable laws, ordinances, rules,
regulations and orders of any public body having jurisdiction. He will erect and
maintain, as required by the conditions and progress of the WORK, all necessary
safeguards for safety and protection. He will notify owners of adjacent utilities when
prosecution of the WORK may affect them. The CONTRACTOR will remedy all
damage, injury or loss to any property caused, directly or indirectly, in whole or in
part, by the CONTRACTOR, any SUBCONTRACTOR or anyone directly or
indirectly employed by any of them or anyone for whose acts any of them may be
liable, except damage or loss attributable to the fault of the CONTRACT
DOCUMENTS or to the acts or omissions of the OWNER or the ENGINEER or
anyone employed by either of them or anyone for whose acts either of them may be
liable, and not attributable, directly or indirectly, in whole or in part, to the fault or
negligence of the CONTRACTOR.

GC-11.03 In emergencies affecting the safety of persons or the WORK or property at the site
or adjacent thereto, the CONTRACTOR, without special instruction or authorization
from the ENGINEER or OWNER, shall act to prevent threatened damage, injury or
loss. He will give the ENGINEER prompt WRITTEN NOTICE of any significant
changes in the WORK or deviations from the CONTRACT DOCUMENTS caused
thereby, and a CHANGE ORDER shall thereupon be issued covering the changes
and deviations involved.
SUPERVISION BY CONTRACTOR  

GC-12.01 Thru 13.01

GC-12  SUPERVISION BY CONTRACTOR

The CONTRACTOR will supervise and direct the WORK. He will be solely responsible for the means, methods, techniques, sequences and procedures of construction. The CONTRACTOR will employ and maintain on the WORK a qualified supervisor or superintendent who shall have been designated in writing by the CONTRACTOR as the CONTRACTOR’S representative at the site. The supervisor shall have full authority to act on behalf of the CONTRACTOR and all communications given to the supervisor shall be as binding as if given to the CONTRACTOR. The supervisor shall be present on the site at all times as required to perform adequate supervision and coordination of the WORK.

CHANGES IN THE WORK  

GC-13.01 Thru 13.02

GC-13  CHANGES IN THE WORK

GC-13.01  The OWNER may at any time, as the need arises, order changes within the scope of the WORK without invalidating the Agreement. If such changes increase or decrease the amount due under the CONTRACT DOCUMENTS, or in the time required for performance of the WORK; an equitable adjustment shall be authorized by CHANGE ORDER.

GC-13.02  The ENGINEER, also, may at any time, by issuing a FIELD ORDER, make changes in the details of the WORK. The CONTRACTOR shall proceed with the performance of any changes in the WORK so ordered by the ENGINEER unless the CONTRACTOR believes that such FIELD ORDER entitles him to a change in CONTRACT PRICE or TIME, or both, in which event he shall give the ENGINEER WRITTEN NOTICE thereof within seven (7) days after the receipt of the ordered change. Thereafter the CONTRACTOR shall document the basis for the change in CONTRACT PRICE or TIME within thirty (30) days. The CONTRACTOR shall not execute such changes pending the receipt of an executed CHANGE ORDER or further instructions from the OWNER.

CHANGES IN CONTRACT PRICE  

GC-14.01 Thru 14.01

GC-14  CHANGES IN CONTRACT PRICE

GC-14.01  The CONTRACT PRICE may be changed only by a CHANGE ORDER. The value of any WORK covered by a CHANGE ORDER or of any claim for increase or decrease in the CONTRACT PRICE shall be determined by one or more of the following methods in the order of precedence listed below:

(a)  Unit prices previously approved.
(b)  An agreed lump sum.
(c)  The actual cost for labor, direct overhead, materials, supplies,
equipment, and other services necessary to complete the work. In addition there shall be added an amount to be agreed upon but not to exceed fifteen percent (15%) of the actual cost of the WORK to cover the cost of general overhead and profit.

TIME FOR COMPLETION AND LIQUIDATED DAMAGES GC-15.01 Thru 15.4.3

GC-15 TIME FOR COMPLETION AND LIQUIDATED DAMAGES

GC-15.01 The date of beginning and the time for completion of the WORK are essential conditions of the CONTRACT DOCUMENTS and the WORK embraced shall be commenced on a date specified in the NOTICE TO PROCEED.

GC-15.02 The CONTRACTOR will proceed with the WORK at such rate of progress to insure full completion within the CONTRACT TIME. It is expressly understood and agreed, by and between the CONTRACTOR and the OWNER, that the CONTRACT TIME for the completion of the WORK described herein is a reasonable time, taking into consideration the average climatic and economic conditions and other factors prevailing in the locality of the WORK.

GC-15.03 If the CONTRACTOR shall fail to complete the WORK within the CONTRACT TIME, or extension of time granted by the OWNER, then the CONTRACTOR will pay to the OWNER the amount of liquidated damages as specified in the BID for each calendar day that the CONTRACTOR shall be in default after the time stipulated in the CONTRACT DOCUMENTS.

GC-15.04 The CONTRACTOR shall not be charged with liquidated damages or any excess cost when the delay in completion of the WORK is due to the following, and the CONTRACTOR has promptly given WRITTEN NOTICE of such delay to the OWNER or ENGINEER.

15.04.1 To any preference, priority or allocation order duly issued by the OWNER.
15.04.2 To unforeseeable causes beyond the control and without the fault or negligence of the CONTRACTOR, including but not restricted to, acts of God, or of the public enemy, acts of the OWNER, acts of another CONTRACTOR in the performance of a contract with the OWNER, fires, floods, epidemics, quarantine restrictions, strikes, freight embargoes, and abnormal and unforeseeable weather; and
15.04.3 To any delays of SUBCONTRACTORS occasioned by any of the causes specified in paragraphs 15.4.1 and 15.4.2 of this article.

CORRECTION OF WORK GC-16.01 Thru 16.02

GC-16 CORRECTION OF WORK

GC-16.01 The CONTRACTOR shall promptly remove from the premises all WORK rejected by the ENGINEER for failure to comply with the CONTRACT DOCUMENTS, whether incorporated in the construction or not, and the CONTRACTOR shall promptly replace and re-execute the WORK in accordance with the CONTRACT
DOCUMENTS and without expense to the OWNER and shall bear the expense of making good all WORK of other CONTRACTORS destroyed or damaged by such removal or replacement.

GC-16.02 All removal and replacement WORK shall be done at the CONTRACTOR’S expense. If the CONTRACTOR does not take action to remove such rejected WORK within ten (10) days after receipt of WRITTEN NOTICE, the OWNER may remove such WORK and store the materials at the expense of the CONTRACTOR.

SUBSURFACE CONDITIONS GC-17.01 Thru 17.02

GC-17 SUBSURFACE CONDITIONS

GC-17.01 The CONTRACTOR shall promptly, and before such conditions are disturbed, except in the event of an emergency, notify the OWNER by WRITTEN NOTICE of:

17.01.1 Subsurface or latent physical conditions at the site differing materially from those indicated in the CONTRACT DOCUMENTS; or

17.01.2 Unknown physical conditions at the site, of an unusual nature, differing materially from those ordinarily encountered and generally recognized as inhering in WORK of the character provided for in the CONTRACT DOCUMENTS.

GC-17.02 The OWNER shall promptly investigate the conditions, and if he finds that such conditions do so materially differ and cause an increase or decrease in the cost of, or in the time required for, performance of the WORK an equitable adjustment shall be made and the CONTRACT DOCUMENTS shall be modified by a CHANGE ORDER. Any claim of the CONTRACTOR for adjustment hereunder shall not be allowed unless he has given the required WRITTEN NOTICE; provided that the OWNER may, if he determined the facts so justify, consider and adjust any such claims asserted before the date of final payment.
SUSPENSION OF WORK, TERMINATION AND DELAY  GC-18.01 Thru 18.02

GC-18  SUSPENSION OF WORK, TERMINATION AND DELAY

GC-18.01 The OWNER may, at any time and without cause, suspend the WORK or any portion thereof for a period of not more than ninety (90) days or such further time as agreed upon by the CONTRACTOR, by WRITTEN NOTICE to the CONTRACTOR and the ENGINEER which notice shall fix the date on which WORK shall be resumed. The CONTRACTOR will resume that WORK on the date so fixed. The CONTRACTOR will be allowed an increase in the CONTRACT PRICE or an extension of the CONTRACT TIME, or both, directly attributable to any suspension.

GC-18.02 If the CONTRACTOR is adjudged a bankrupt or insolvent, or if he makes a general assignment for the benefit of his creditors, or if a trustee or receiver is appointed for the CONTRACTOR or for any of his property, or if he files a petition to take advantage of any debtor’s act, or to recognize under the bankruptcy or applicable laws, or if he repeatedly fails to supply sufficient skilled workmen or suitable materials or equipment, or if he repeatedly fails to make prompt payment to SUBCONTRACTORS or for labor, materials or equipment or if he disregards laws, ordinances, rules, regulations or orders of any public body having jurisdiction of the WORK or if he disregards the authority of the ENGINEER, or if he otherwise violates any provision of the CONTRACT DOCUMENTS, then the OWNER may, without prejudice to any other right or remedy and after giving the CONTRACTOR and his surety a minimum of ten (10) days from delivery of a WRITTEN NOTICE, terminate the services of the CONTRACTOR and take possession of the PROJECT and of all materials, equipment, tools, construction equipment and machinery thereon owned by the CONTRACTOR and finish the WORK by whatever method he may deem expedient. In such case the CONTRACTOR shall not be entitled to receive any further payment until the WORK is finished. If the unpaid balance of the CONTRACT PRICE exceeds the direct and indirect costs of completing the PROJECT, including compensation for additional professional services, such excess shall be paid to the CONTRACTOR. If such costs exceed such unpaid balance, the CONTRACTOR will pay the difference to the OWNER. Such costs incurred by the OWNER will be determined by the ENGINEER and incorporated in a CHANGE ORDER.

GC-18.03 Where the CONTRACTOR’S services have been so terminated by the OWNER, said termination shall not affect any right of the OWNER against the CONTRACTOR then existing or which may thereafter accrue. Any retention or payment of monies by the OWNER due the CONTRACTOR will not release the
CONTRACTOR from compliance with the CONTRACT DOCUMENTS.

GC-18.04 After ten (10) days from delivery of WRITTEN NOTICE to the CONTRACTOR and the ENGINEER, the OWNER may, without cause and without prejudice to any other right or remedy, elect to abandon the PROJECT and terminate the Contract. In such case, the CONTRACTOR shall be paid for all WORK executed and any expense sustained plus reasonable profit.

GC-18.05 If, through no act or fault of the CONTRACTOR, the WORK is suspended for a period of more than ninety (90) days by the OWNER or under an order of court or other public authority, or the ENGINEER fails to act on any request for payment within thirty (30) days after it is submitted, or the OWNER fails to pay the CONTRACTOR substantially the sum approved by the ENGINEER or awarded by arbitrators within thirty (30) days of its approval and presentation, then the CONTRACTOR may, after ten (10) days from delivery of a WRITTEN NOTICE to the OWNER and the ENGINEER, terminate the CONTRACT and recover from the OWNER payment for all WORK executed and all expenses sustained. In addition and in lieu of terminating the CONTRACT, if the ENGINEER has failed to act on a request for payment or if the OWNER has failed to make any payment as aforesaid, the CONTRACTOR may upon ten (10) days notice to the OWNER and the ENGINEER stop the WORK until he has been paid all amounts then due, in which event and upon resumption of the WORK, CHANGE ORDERS shall be issued for adjusting the CONTRACT PRICE or extending the CONTRACT TIME or both to compensate for the costs and delays attributable to the stoppage of the WORK.

GC-18.06 If the performance of all or any portion of the WORK is suspended, delayed, or interrupted as a result of a failure of the OWNER or ENGINEER to act within the time specified in the CONTRACT DOCUMENTS, or if no time is specified, within a reasonable time, an adjustment in the CONTRACT PRICE or an extension of the CONTRACT TIME, or both, shall be made by CHANGE ORDER to compensate the CONTRACTOR for the costs and delays necessarily caused by the failure of the OWNER or ENGINEER.

PAYMENT TO CONTRACTOR  GC-19.01 Thru 19.01

GC-19  PAYMENT TO CONTRACTOR

GC-19.01 At least ten (10) days before each progress payments falls due (but not more often than once a month), the CONTRACTOR will submit to the ENGINEER a partial payment estimate filled out and signed by the CONTRACTOR covering the WORK performed during the period covered by the partial payment estimate and supported by such data as the ENGINEER may reasonably require. If payment is requested on the basis of materials and equipment not incorporated in the WORK but delivered and suitably stored at or near the site, the partial payment estimate shall also be accompanied by such supporting data, satisfactory to the OWNER, as will
establish the OWNER'S title to the material and equipment and protect his interest therein, including applicable insurance. The ENGINEER will, within ten (10) days after receipt of each partial payment estimate, either indicate in writing his approval of payment and present partial payment estimate to the OWNER, or return the partial payment estimate to the CONTRACTOR indicating in writing his reasons to approve payment. In the latter case, the CONTRACTOR may make the necessary corrections and resubmit the partial payment estimate. The OWNER will within fifteen (15) days of presentation to him of an approved partial payment estimate, pay the CONTRACTOR a progress payment on the basis of the approved partial payment estimate less the retainage. The retainage shall be an amount equal to five percent (5%) of said estimate of the WORK completed.

Upon substantial completion of the WORK, any amount retained may be paid to the CONTRACTOR. When the WORK has been substantially completed except for work which cannot be completed because of weather conditions, lack of materials or other reasons which in the judgment of the OWNER are valid reasons for noncompletion, the OWNER may make additional payments, retaining at all times an amount sufficient to cover the estimated cost of the WORK still to be completed.

GC-19.02 The request for payment may also include an allowance for the cost of such major materials and equipment, which are suitably stored either at or near the site.

GC-19.03 Prior to SUBSTANTIAL COMPLETION, the OWNER, with the approval of the ENGINEER and with the concurrence of the CONTRACTOR, may use any completed or substantially completed portions of the WORK. Such use shall not constitute an acceptance of such portions of the WORK.

GC-19.04 The OWNER shall have the right to enter the premises for the purpose of doing work not covered by the CONTRACT DOCUMENTS. This provision shall not be construed as relieving the CONTRACTOR of the sole responsibility for the care and protection of the WORK, or the restoration of any damaged WORK except such as may be caused by agents or employees of the OWNER.

GC-19.05 Upon completion and acceptance of the WORK, the ENGINEER shall issue a certificate attached to the final payment request that the WORK has been accepted by him under the conditions of the CONTRACT DOCUMENTS. The entire balance found to be due the CONTRACTOR, including the retained percentages, but except such sums as may be lawfully retained by the OWNER shall be paid to the CONTRACTOR within thirty (30) days of completion and acceptance of the WORK.

GC-19.06 The CONTRACTOR will indemnify and save the OWNER or the OWNER’S agents harmless from all claims growing out of the lawful demand of SUBCONTRACTORS, laborers, workmen, mechanics, material men, and
furnishers of machinery and parts thereof, equipment, tools, and all supplies, incurred in the furtherance of the performance of the WORK. The CONTRACTOR shall, at the OWNER’S request, furnish satisfactory evidence that all obligations of the nature designated above have been paid, discharged, or waived. If the CONTRACTOR fails to do so the OWNER may, after having notified the CONTRACTOR, either pay unpaid bills or withhold from the CONTRACTOR’S unpaid compensation a sum of money deemed reasonably sufficient to pay any and all such lawful claims until satisfactory evidence is furnished that all liabilities have been fully discharged whereupon payment to the CONTRACTOR shall be resumed in accordance with the terms of the CONTRACT DOCUMENTS, but in no event shall the provisions of this sentence be construed to impose any obligations upon the OWNER to either the CONTRACTOR, his Surety, or any third party. In paying any unpaid bills of the CONTRACTOR, any payment so made by the OWNER shall be considered as a payment made under the CONTRACT DOCUMENTS by the OWNER to the CONTRACTOR and the OWNER shall not be liable to the CONTRACTOR for any such payment made in good faith.

GC-19.07 If the OWNER fails to make payment thirty (30) days after approval by the ENGINEER, in addition to other remedies available to the CONTRACTOR, there shall be added to each such payment interest at the maximum legal rate commencing on the first day after said payment is due and continuing until the payment is received by the CONTRACTOR.

ACCEPTANCE OF FINAL PAYMENT AS RELEASE GC-20.01 Thru 20.01

GC-20 ACCEPTANCE OF FINAL PAYMENT AS RELEASE
GC-20.01 The acceptance by the CONTRACTOR of final payment shall be and shall operate as a release to the OWNER of all claims and all liability to the CONTRACTOR other than claims in stated amounts as may be specifically excepted by the CONTRACTOR for all things done or furnished in connection with this WORK and for every act and neglect of the OWNER and others relating to or arising out of this WORK. Any payment, however, final or otherwise, shall not release the CONTRACTOR or his sureties from any obligations under the CONTRACT DOCUMENTS or the Performance Bond and Payment Bonds.

INSURANCE GC-21.01 Thru 21.03

GC-21 INSURANCE
GC-21.01 The CONTRACTOR shall purchase and maintain such insurance as will protect him from claims set forth below which may arise out of or result from the CONTRACTOR’S executions of the WORK, whether such execution be by himself or by any SUBCONTRACT or by anyone directly employed by any of them, or by anyone for whose acts any of them may be liable.
21.01.1 Claims under workmen's compensation, disability benefit and other similar employee benefit acts;

21.01.2 Claims for damages because of bodily injury, occupational sickness or disease, or death of his employees;

21.01.3 Claims for damages because of bodily injury, sickness or disease, or death of any person other than his employees;

21.01.4 Claims for damages insured by usual personal injury liability coverage which are sustained (1) by any person as a result of an offense directly or indirectly related to the employment of such person by the CONTRACTOR, or (2) by any other person; and

21.01.5 Claims for damages because of injury to or destruction of tangible property, including loss of use resulting therefrom.

GC-21.02 Certificates of insurance acceptable to the OWNER shall be filed with the OWNER prior to commencement of the WORK. These Certificates shall contain a provision that coverages afforded under the policies will not be cancelled unless at least fifteen (15) days prior WRITTEN NOTICE has been given to the OWNER.

GC-21.03 The CONTRACTOR shall procure and maintain, at his own expense, during the CONTRACT TIME, liability insurance as hereinafter specified;

21.03.1 CONTRACTOR’S General Public Liability and Property Damage Insurance including vehicle coverage issued to the CONTRACTOR and protecting him from all claims for personal injury, including death, and all claims for destruction of our damage to property, arising out of or in connection with any operations under the CONTRACT DOCUMENTS, whether such operations be by himself or by any SUBCONTRACTOR under him, or anyone directly or indirectly employed by the CONTRACTOR or by a SUBCONTRACTOR under him. Insurance shall be written with the following limits of liability:

General Aggregate $2,000,000
Products/Completed Operations Aggregate $2,000,000
Per Occurrence $2,000,000
Fire Legal Liability $500,000
Medical Payments $5,000

21.03.2 The CONTRACTOR shall acquire and maintain, if applicable, Fire and Extended Coverage insurance upon the PROJECT to the full insurable value thereof for the benefit of the OWNER, the CONTRACTOR, and SUBCONTRACTORS as their interest may appear. This provision shall in no way release the CONTRACTOR or CONTRACTOR’S surety from obligations under the CONTRACT DOCUMENTS to fully complete the PROJECT.
The CONTRACTOR shall procure and maintain, at his own expense, during the CONTRACT TIME, in accordance with the provisions of the laws of the state in which the work is performed, Workmen's Compensation Insurance, including occupational disease provisions, for all of his employees at the site of the PROJECT and in case any work is sublet, the CONTRACTOR shall require such SUBCONTRACTOR similarly to provide Workmen's Compensation Insurance, including occupational disease provisions for all of the latter’s employees unless such employees are covered by the protection afforded by the CONTRACTOR. In case any class of employees engaged in hazardous work under this contract at the site of the PROJECT is not protected under Workmen’s Compensation statute, the CONTRACTOR shall provide, and shall cause each SUBCONTRACTOR to provide, adequate and suitable insurance for the protection of his employees not otherwise protected.

The CONTRACTOR shall secure, if applicable, “All Risk” type Builders Risk Insurance for WORK to be performed. Unless specifically authorized by the OWNER, the amount of such insurance shall not be less than the CONTRACT PRICE totaled in the bid. The policy shall cover not less than the losses due to fire, explosion, hail, lightning, vandalism, malicious mischief, win, collapse, riot, aircraft, and smoke during the CONTRACT TIME, and until the WORK is accepted by the OWNER. The policy shall name as the insured the CONTRACTOR, the ENGINEER, and the OWNER.

The CONTRACTOR shall within ten (10) days after the receipt of the NOTICE OF AWARD furnish the OWNER with a Performance Bond and a Payment Bond in the penal sums equal to the amount of the CONTRACT PRICE, conditioned upon the performance by the CONTRACTOR of all undertakings, covenants, terms, conditions and agreements of the CONTRACT DOCUMENTS, and upon the prompt payment by the CONTRACTOR to all persons supplying labor and materials in the prosecution of the WORK provided by the CONTRACT DOCUMENTS. Such BONDS shall be executed by the CONTRACTOR and a corporate bonding company licensed to transact such business in the state in which the WORK is to be performed and named on the current list of "Surety Companies Acceptable on Federal Bonds" as published in the Treasury Department Circular Number 570. The expense of these BONDS shall be borne by the CONTRACTOR. If at any time a surety on any such BOND is declared bankrupt or loses its right to do business in the state in which the WORK is to be performed or is removed from the list of Surety Companies accepted on Federal BONDS,
CONTRACTOR shall within ten (10) days after notice from the OWNER to do so, substitute an acceptable BOND (or BONDS) in such form and sum and signed by such other surety or sureties as may be satisfactory to the OWNER. The premiums on such BOND shall be paid by the CONTRACTOR. No further payments shall be deemed due nor shall be made until the new surety or sureties shall have furnished an acceptable BOND to the OWNER.

ASSIGNMENTS GC-23.01 Thru 23.01

Neither the CONTRACTOR nor the OWNER shall sell, transfer, assign or otherwise dispose of the Contract or any portion thereof, or of his right, title or interest therein, or his obligations thereunder, without written consent of the other party.

INDEMNIFICATION GC-24.01 Thru 24.03

The CONTRACTOR will indemnify and hold harmless the OWNER and the ENGINEER and their agents and employees from and against all claims, damages, losses and expenses including attorneys’ fees arising out of or resulting from the performance of the WORK, provided that any such claims, damage, loss or expense is attributable to bodily injury, sickness, disease or death, or injury to or destruction of tangible property, including the loss of use resulting there from; and is caused in whole or in part by any negligent or willful act or omission of the CONTRACTOR, and SUBCONTRACTOR, anyone directly or indirectly employed by any of them or anyone for whose acts any of them may be liable.

In any and all claims against the OWNER or the ENGINEER, or any of their agents or employees, by any employee of the CONTRACTOR, and SUBCONTRACTOR, anyone directly or indirectly employed by any of them, or anyone for whose acts any of them may be liable, the indemnification obligation shall not be limited in any way by any limitation on the amount or type of damages, compensation or benefits payable by or for the CONTRACTOR or any SUBCONTRACTOR under workmen’s compensation acts, disability benefit acts or other employee benefits acts.

The obligation of the CONTRACTOR under this paragraph shall not extend to the liability of the ENGINEER, his agents or employees arising out of the preparation or approval of maps, DRAWINGS, opinions, reports, surveys, CHANGE ORDERS, designs or SPECIFICATIONS.

SEPARATE CONTRACTS GC-25.01 Thru 25.03

The OWNER reserves the right to let other contracts in connection with this PROJECT. The CONTRACTOR shall afford other CONTRACTORS reasonable
opportunity for the introduction and storage of their materials and the execution of their WORK, and shall properly connect and coordinate his WORK with theirs. If the proper execution or results of any part of the CONTRACTOR'S WORK depends upon the WORK of any other CONTRACTOR, the CONTRACTOR shall inspect and promptly report to the ENGINEER any defects in such WORK that render it unsuitable for such proper execution and results.

GC-25.02 The OWNER may perform additional WORK related to the PROJECT by himself, or he may let other contracts containing provisions similar to these. The CONTRACTOR will afford the other CONTRACTORS who are parties to such Contracts (or the OWNER, if he is performing the additional WORK himself), reasonable opportunity for the introduction and storage of materials and equipment and the execution of WORK, and shall properly connect and coordinate his WORK with theirs.

GC-25.03 If the performance of additional WORK by other CONTRACTORS or the OWNER is not noted in the CONTRACT DOCUMENTS prior to the execution of the CONTRACT, written notice thereof shall be given to the CONTRACTOR prior to starting any such additional WORK. If the CONTRACTOR believes that the performance of such additional WORK by the OWNER or others involves him in additional expense or entitles him to an extension of the CONTRACT TIME, he may make a claim thereof as provided in Sections 14 and 15.

SUBCONTRACTING GC-26.01 Thru 26.05

GC-26 SUBCONTRACTING

GC-26.01 The CONTRACTOR may utilize the services of specialty SUBCONTRACTORS on those parts of the WORK which, under normal contracting practices, are performed by specialty CONTRACTORS.

GC-26.02 The CONTRACTOR shall not award WORK to SUBCONTRACTOR(S), in excess of fifty percent (50%) of the CONTRACT PRICE, without prior written approval of the OWNER.

GC-26.03 The CONTRACTOR shall be fully responsible to the OWNER for the acts and omissions of his SUBCONTRACTORS, and of persons either directly or indirectly employed by them, as he is for the acts and omissions of persons directly employed by him.

GC-26.04 The CONTRACTOR shall cause appropriate provisions to be inserted in all subcontracts relative to the WORK to bind SUBCONTRACTORS to the CONTRACTOR by the terms of the CONTRACT DOCUMENTS insofar as applicable to the WORK of SUBCONTRACTORS and to give the CONTRACTOR the same power as regards terminating any subcontract that the OWNER may exercise over the CONTRACTOR under any provision of the CONTRACT DOCUMENTS.
GC-26.05 Nothing contained in this CONTRACT shall create any contractual relation between any SUBCONTRACTOR and the OWNER.

ENGINEERING AUTHORITY GC-27.01 Thru 27.04

GC-27 ENGINEERS AUTHORITY

GC-27.01 The ENGINEER shall act as the OWNER'S representative during the construction period. He shall decide questions, which may arise as to quality and acceptability of materials furnished and WORK performed. He shall interpret the intent of the CONTRACT DOCUMENTS in a fair and unbiased manner. The ENGINEER will make visits to the site and determine if the WORK is proceeding in accordance with the CONTRACT DOCUMENTS.

GC-27.02 The CONTRACTOR will be held strictly to the intent of the CONTRACT DOCUMENTS in regard to the quality of materials, workmanship and execution of the WORK. Inspections may be made at the factory or fabrication plant of the source of material supply.

GC-27.03 The ENGINEER will not be responsible for the construction means, controls, techniques, sequences, procedures, or construction safety.

GC-27.04 The ENGINEER shall promptly make decisions relative to interpretation of the CONTRACT DOCUMENTS.

LAND AND RIGHTS-OF-WAY GC-28.01 Thru 28.03

GC-28 LAND AND RIGHTS-OF-WAY

GC-28.01 Prior to issuance of NOTICE TO PROCEED, the OWNER shall obtain all land and rights-of-way necessary for carrying out and for the completion of the WORK to be performed pursuant to the CONTRACT DOCUMENTS, unless otherwise mutually agreed.

GC-28.02 The OWNER shall provide to the CONTRACTOR information, which delineates and describes the lands owned and right-of-way acquired.

GC-28.03 The CONTRACTOR shall provide at his own expense and without liability to the OWNER any additional land and access thereto that the CONTRACTOR may desire for temporary construction facilities, or for storage of materials.

GUARANTY GC-29.01 Thru 19.01

GC-29 GUARANTY

GC-29.01 The CONTRACTOR shall guarantee all materials and equipment furnished and WORK performed for a period of one (1) year from the date of SUBSTANTIAL COMPLETION. The CONTRACTOR warrants and guarantees for a period of one (1) year from the date of SUBSTANTIAL COMPLETION of the system that the completed system is free from all defects due to fault materials or workmanship and the CONTRACTOR shall promptly make such corrections as may be necessary by reason of such defects including the repairs of any damage to other
parts of the system resulting from such defects. The OWNER will give notice of observed defects with reasonable promptness. In the event that the CONTRACTOR should fail to make such repairs, adjustments, or other WORK that may be made necessary by such defects, the OWNER may do so and charge the CONTRACTOR the cost thereby incurred. The Performance Bond shall remain in full force and effect through the guarantee period.

**TAXES** GC-30.01 Thru 30.01

- **GC-30** TAXES
- **GC-30.01** The CONTRACTOR will pay all sales, consumer, use and other similar taxes required by the law of the place where the WORK is performed.
SPECIAL CONDITIONS

1. **QUALIFICATIONS OF BIDDER**
The apparent low bidder shall submit to the **OWNER** a list and description of work performed on previous projects similar to this along with evidence of financial ability, including a list of equipment owned, to satisfactorily complete the project, if requested by the **OWNER**.

2. **SUBCONTRACTORS AND SUPPLIERS**
In accordance with paragraph GC-26 of the **General Conditions** the **CONTRACTOR** shall submit a list of any Subcontractors and major material suppliers proposed on this project.

3. **CONTRACT SECURITY**
Contract Security shall be provided as set out in the **Information for Bidders** and in accordance with paragraph GC-22 of the **General Conditions**.

4. **CONTRACTOR’S AND SUBCONTRACTOR’S PUBLIC LIABILITY, AUTOMOBILE LIABILITY AND PROPERTY DAMAGE INSURANCE**
With reference to Section GC-21 of the **General Conditions**, the **CONTRACTOR** is advised that he shall purchase and maintain at his own expense Property Insurance as will protect the **CONTRACTOR** and the **OWNER** from loss or damage while the project is under construction and prior to full acceptance thereof by the **OWNER**.

5. **ESTIMATE FOR PARTIAL PAYMENT**
Form FHA 424-18 "Partial Payment Estimate", shall be used when estimating periodic payment due the **CONTRACTOR**. The applications for progress or final payments by the **CONTRACTOR** will be submitted to the **ENGINEER** on or before the 5th day of each calendar month. The date at which receipt of partial payment by **ENGINEER** as stipulated in **General Conditions** (GC-19) is hereby set as the 5th day of the month provided estimates are received by such time. The partial payment estimate shall be for work performed no later than the last day of the preceding calendar month.

6. **CONTRACTOR - WITHDRAWAL OF RETAINED FUNDS**
The **GENERAL CONTRACTOR**, subcontractor and material suppliers waive all rights to withdrawal of retained funds, which may accrue under Tennessee Code Annotated 12-434.
TECHNICAL SPECIFICATIONS
TOWN OF SMYRNA
SEWER LINE ADDITIONS

SECTION 1 - SCOPE OF PROJECT

1.01 GENERAL

The CONTRACTOR shall furnish all materials, equipment, machinery, labor, etc., necessary for the construction of the facilities more particularly described elsewhere in the specifications and shown on the drawings.

The CONTRACTOR shall perform all necessary clearing, staking, excavating, backfilling, grading, cleanup, restoration of damage to property, testing, etc., for the proper and complete installation of the system and restoration of the surface to its original condition.

1.02 WORK AREA AND ORDER OF WORK

The CONTRACTOR shall prepare and submit a detailed schedule showing this proposed sequence of work on the project for discussion at the Pre-Construction Meeting. The CONTRACTOR shall coordinate the work schedule with the OWNER so as to disrupt traffic and sewer line service as little as possible. The schedule of work shall be approved by the ENGINEER.

1.03 CONTRACTOR QUALIFICATIONS

The CONTRACTOR must have the proper equipment and qualified personnel to accomplish the work required. He must be prepared to provide the ENGINEER with satisfactory evidence that: (a) he has completed similar work with similar equipment and materials on at least five (5) previous projects, or (b) his crews and equipment can perform satisfactorily as established by actual demonstration to the ENGINEER.

Failure to perform the work satisfactorily shall be grounds to cancel the contract and for the OWNER to proceed in whatever manner available to satisfactorily complete the work.

1.04 SEWAGE BYPASSING

Where sewage flow exceeds the maximum allowance in performance of the various work items, the CONTRACTOR shall provide pumps and bypass pipelines as required to divert any excess flow around the work area. Nevertheless, all sewage must remain in the system. Under no circumstances will the CONTRACTOR be allowed to discharge sewage into natural streams, drainage ditches or other locations that could endanger the public health, violate laws and regulations or cause a public nuisance.

1.05 TRAFFIC CONTROL

The CONTRACTOR shall maintain sufficient warning lights, traffic signs, road barriers, traffic cones, flagmen, etc., on or along any or all portions of any street or alley which due to the CONTRACTOR’S operations, are not in their normal
condition for handling vehicular or pedestrian traffic. Traffic is to be maintained on all roads and streets that must be crossed by work operations. The CONTRACTOR has to adhere to all applicable local and state highway regulations regarding traffic control during construction operations including the latest manual of uniform traffic control. There will be no separate pay item for traffic control and all costs thereof shall be included in the costs of the various project bid items.

1.06 **DISPOSAL OF MATERIAL**

The CONTRACTOR shall be responsible for obtaining an area that will be suitable for disposal of all materials removed from the sewers during the cleaning operation.

1.07 **INCREASE OR REDUCTION OF WORK** - The project has a limited budget. In order to insure that the budget is met, the OWNER reserves the right to increase or decrease the quantities of work shown in the BID form to make the project costs conform to the available funds.
SECTION 2 - PRELIMINARY WORK

2.01 LOCATION AND PROTECTION OF UNDERGROUND UTILITIES

Prior to trenching, the CONTRACTOR shall determine insofar as possible, the actual location of all underground utilities possible, the actual location of all underground utilities in the vicinity of his operations and shall clearly mark their location so that they may be avoided by equipment operators. Where such utility lines or services appear to lie in the path of construction they shall be uncovered in advance to determine the exact location and depth and to avoid damage due to trenching operations. Existing facilities shall be protected during construction or removed and replaced in equal condition, as necessary.

Should any existing utility line or service be damaged during, or as a result of the CONTRACTOR'S operations, the CONTRACTOR shall take such emergency measures as may be necessary to minimize damage and shall immediately notify the utility involved. The CONTRACTOR shall then repair the damage to the satisfaction of the utility or shall pay the utility for making the necessary repairs. In all cases, the restoration and/or repair shall be such that the damaged structure will be in as good or better condition as before the damage occurred.

2.02 SURVEYING, STAKING AND CUT SHEETS

The ENGINEER will provide adequate benchmarks and control lines for sewers, but offset staking shall be the responsibility of the CONTRACTOR. The CONTRACTOR shall prepare cut sheets and submit them in quadruplicate for the ENGINEERS approval. Two sets of the approved cut sheets will be returned to the CONTRACTOR.

Cut sheets shall indicate: invert elevation, ground elevation above sewer center line, offset hub elevation, offset hub cut, and offset distances and direction. For sewers laid by batterboard, stringline and gradepole method the offset stations shall be set at points in and out of manholes and at fifty (50) foot minimum for grades of 1% or more, and at twenty five (25) foot minimum for grades less than 1%. For sewers laid by laser offset stations shall be set at points in and out of manholes with one offset station located twenty (20) feet upstream from manhole or an alternate system approved by the ENGINEERS.

2.03 SAFEGUARDING OBSTRUCTIONS

The CONTRACTOR shall be responsible for the removal, safeguarding and replacement of fences, walls, structures, culverts, street signs, private utilities, billboards, shrubs, flowers and small trees, mailboxes or other obstruction which must be restored to at least their original condition. Notification of all required fence cuts shall be given to property owner 48 hours prior to construction on property.

2.04 CLEARING AND GRUBBING

The CONTRACTOR shall be responsible for cutting, removing and disposing of all trees, brush, stumps, roots, and weeds within the construction area. Disposal shall be by means of chippers, landfills, or other approved methods not in conflict with State or local ordinances. Care shall be taken to avoid unnecessary cutting or damage to trees. The CONTRACTOR will be responsible for loss or damage to trees located more than three (3) feet from the sewer centerline.
SECTION 3 - MATERIALS

3.01 GENERAL

All materials to be incorporated in the project shall be first quality, new and undamaged material conforming to all applicable portions of these specifications.

When a material, equipment, or system is specified by the name of one or more manufacturer, such material, equipment, or system shall become an essential element of the Contract. If the CONTRACTOR desires to use another material, equipment, or system in lieu thereof, he shall request approval in writing and shall submit samples and data as required for the ENGINEER’S consideration. The ENGINEER will be the final judge of the acceptability of the substitution. No substitution shall be made without authority in writing from the ENGINEER.

3.02 CEMENT

Cement shall be Portland cement of a brand approved by the ENGINEERS and shall conform to "Standard Specifications for Portland Cement", Type, 1 ASTM Designation C150, latest revision. Cement shall be furnished in undamaged 94 pound, one (1) cubic foot sacks, and shall show no evidence of lumping.

3.03 CONCRETE FINE AGGREGATE

Fine aggregate shall be clean, hard, uncoated natural sand conforming to ASTM Designation C33, latest revision, "Standard Specifications for Concrete Aggregate".

3.04 CONCRETE COARSE AGGREGATE

Coarse aggregate shall consist of clean, hard, dense particles of stone or gravel conforming to ASTM Designation C33, latest revision, "Standard Specifications for Concrete Aggregate". Aggregate shall be well graded between 1-1/2" and #4 sieve sizes.

3.05 WATER

Water used in mixing concrete shall be clean and free from organic matter, pollutants and other foreign materials.

3.06 READY MIX CONCRETE

Ready-mix concrete shall be secured only from a source approved by the ENGINEERS and shall conform to ASTM Designation C94, latest revision, "Specifications for Ready-Mix Concrete". Before any concrete is delivered to the job site, the supplier must furnish a statement of the proportions of cement, fine aggregate and coarse aggregate to be used for each mix ordered, and must receive the ENGINEERS approval of such proportions.
3.07 **CLASS "A" CONCRETE**

Class "A" concrete shall have a minimum compressive strength of 4,000 pounds per square inch in 28 days and shall contain not less than six (6) sacks of cement per cubic yard.

3.08 **CLASS "B" CONCRETE**

Class "B" concrete shall have a minimum compressive strength of 2000 pounds per square inch in 28 days and shall contain not less than 4-1/2 sacks of concrete per cubic yard.

3.09 **METAL REINFORCING**

Reinforcing bars shall be intermediate grade steel conforming to ASTM Designation A15, latest revision, *"Standard Specifications for Billet Steel Bars for Concrete Reinforcement"*. Bars shall be deformed with a cross sectional area at all points equal to that of plain bars of equal nominal size.

3.10 **CRUSHED STONE**

Crushed stone for bedding or backfill shall be Tenn. State Highway Standard size No. 67 and shall meet State Highway Department Standards for road surfacing.

Crushed stone for base shall conform to Section 303, Highway Department Specifications and shall be Class A, Grade D.

3.11 **PEA GRAVEL**

Pea gravel for shaping cradle bedding shall be #4 to 1/2" size Ohio River, or approved local gravel of similar character.

3.12 **MANHOLE FRAMES & COVERS**

1. **General**: Manhole frames and covers shall be gray cast iron conforming to ASTM A48-64, Class 20, unless shown otherwise below, and shall be first quality castings free from blow-holes, shrinkage, distortion or other defects. After cleaning, casting shall be painted with a bituminous coating, giving tough, smooth surface not tacky or having tendency to scale or "alligator". Frames and covers shall be as shown on Detail Drawings. Unless shown otherwise on Plans, covers to be solid with words "SANITARY SEWER" cast in cover, with pick hole cast in cover. Frames and covers for traffic conditions shall have machined contact surfaces to prevent rocking.

2. **Standard Manhole Frames and Covers**: Manhole frames shall be furnished and set in a bed of mastic and grouted into the concrete manhole. The standard frame and cover shall be traffic type of gray cast iron ASTM Designation A 48-64 with a 24-inch diameter opening weighing not less than 400 pounds as shown on the Plans and unless otherwise specified shall be a John Bouchard & Sons Co. No. 1150 or approved equal. The covers shall be the solid self-sealing type with no holes except watertight pick notches or a heavy lifting ring. The surface between the cover and frame shall fit smoothly without rocking and shall be thoroughly
cleaned. Special attention shall be given to insure the proper installation of the rubber gasket in the self-sealing cover. The gasket shall have at least 1/4-inch diameter cross-section. The frame shall be grouted in and fixed directly to the manhole barrel by so as to constitute a watertight seal between the barrel and the frame.

3. **Watertight Manholes Frames & Covers:** The manhole frames shall be set in the same manner prescribed for standard frames except special attention shall be paid to securing a watertight connection to the manhole barrel.

The watertight manhole frame and cover shall be a traffic type of gray cast iron ASTM Designation A 48-64 with a twenty-four inch (24") diameter minimum clear opening weighing not less than 450 pounds and shall be a **John Bouchard & Sons Co. No. 1123 or approved equal** unless otherwise specified on the plans.

The surface cover shall be the solid type with no holes except watertight pick notches or a heavy lifting ring. The surface between this cover and frame shall fit without rocking. The inner cover shall be of the solid type with no holes, shall have not less than two (2) lifting handles and shall have a neoprene sealing gasket at least 7/16-inch diameter cross-section with a hollow center. The inner cover shall be mechanically sealed by means of a removable metal bar located over the inner cover with a centrally located bronze or stainless steel tightening bolt. This bolt shall have a tee-handle or benthandle for turning. The bolt shall have appropriate reinforcing ribs to prevent cracking or distortion when tightened. The inner cover shall have sufficient clearance to allow easy removal from the frame. The frame shall be attached to the manhole barrel by means of four (4) 5/8-inch anchor bolts and shall be set in a bed of mastic so as to constitute a watertight seal between the barrel and frame. Watertight manholes shall be vented at 1000 foot intervals.

3.13 **MANHOLE STEPS**

Manhole steps shall be made of copolymer polypropylene plastic meeting the latest revision of ASTM 2146-68, Type II, Grade 16906 and shall have a 1/2-inch diameter Grade 60 reinforcing rod meeting the latest revision of ASTM Designation A-615 through its center.

Each step shall be twelve inches (12") in width and capable of carrying a load of 1,000 pounds in the center of the step when projected six inches (6") from the wall. Each step shall be equipped with non-skid grooves.

3.14 **MORTAR MATERIALS**

Mortar for manholes shall consist of one part of Portland cement to two parts of sand. Sand shall be a clean natural river sand. When dry 100% of the sand shall pass a #8 sieve and not more than 35% shall pass a #50 sieve.

3.15 **PRECAST CONCRETE MANHOLES**

In order to prevent excessive leakage of water into manholes, special care is warranted in the design and construction of manholes, therefore, this design requires high quality watertight precast manholes. Special emphasis is placed on the connection of the pipeline to the manhole in such a manner as to preclude
shearing and/or leakage. Connection of pipeline to manhole shall be with approved rubber boot cast in manhole. Manholes shall have an inside diameter of 4'-0". Precast concrete manholes shall conform to ASTM Designation C-478, latest revision.

3.16 **CONCRETE SEWER PIPE - REINFORCED**

Reinforced concrete sewer pipe shall not be allowed as an acceptable material.

3.17 **CAST IRON PIPE**

Cast iron pipe shall be manufactured in accordance with ASA Standard A21.6 or A21.8 for centrifugally cast iron pipe. The pipe shall be manufactured of iron having 21/45, or stronger, metal characteristics. Class of pipe shall be as indicated on the drawings or in the absence of specified class shall be Class 22, with minimum thickness as follows:

- 6"..................0.38" Wall Thickness
- 8"..................0.41" Wall Thickness
- 10"...............0.44" Wall Thickness
- 12"...............0.48" Wall Thickness

Pipe shall be furnished in lengths of 18' to 20' and unless otherwise indicated shall be provided with a compression type slip joint equal to Fastite joint as manufactured by American, Super Beli-Tite as manufactured by Clow, or equal.

Pipe shall be furnished with standard thickness cement lining on the inside with a bituminous seal coat and a bituminous coating on the outside. Cement lining shall conform to ASA Standard A21.4. The exterior or the pipe shall be clearly marked to indicate the manufacturer, date of manufacture, pipe class and weight.

3.18 **DUCTILE IRON PIPE**

Ductile iron pipe shall be manufactured in accordance with ASA Standard A21.51 for centrifugally cast ductile iron pipe. The pipe shall be manufactured of iron having acceptance values of 60-42-10. Pipe shall be as indicated on the BID Proposal or shall be at least minimum. Wall thickness for Class 52 pipe shall be as follows:

- 8"..................0.33"
- 10".................0.35"
- 12".................0.37"
- 14".................0.39"
- 16".................0.40"
- 18".................0.41"

Pipe shall be furnished in lengths of 18' to 20' and unless otherwise indicated shall be provided with a compression type slip joint equal to the Fastite joint as manufactured by American. Gaskets and lubricant shall be furnished with the pipe.

Pipe shall be furnished with standard thickness cement lining on the inside with a bituminous seal coat and a bituminous coating on the outside. Cement lining shall conform to ASA Standard A21.4. The exterior of the pipe shall be clearly marked to indicate the manufacturer, date of manufacture, the pipe class and weight. Exterior markings shall also positively identify the pipe as being Ductile Iron.
3.19 **CAST IRON FITTINGS**

All cast iron fittings shall be cement lined, bituminous coated manufactured in accordance with ASA Standard A21.10-1964. Fittings shall be furnished with mechanical joints conforming to ASA A21.11-1964, unless otherwise indicated or directed.

3.20 **SEWER FITTINGS AND ADAPTERS**

Fittings and adapters for use with sewer pipe shall be manufactured to be compatible with piping and pipe joints. Fitting and adapter engineering data shall be submitted to the ENGINEER for approval.

3.21 **POLYVINYL CHLORIDE (PVC) SEWER PIPE**

PVC sewer pipe shall be manufactured of Polyvinyl chloride material as defined and described in ASTM D-1784 and shall be solid wall conforming to ASTM D-3034. For depths 0-3 feet PVC pipe is not allowed. For depths 3-15’ PVC pipe shall be SDR 35. For depths over 15’ PVC will not be allowed.

Joints shall be of bell and spigot type. The bell shall contain and elastomeric gasket which is firmly retained. Solvent weld joints will not be permitted except in an emergency situation when approved by the ENGINEER.

Fittings and plugs shall be supplied by pipe suppliers with equivalent joints. Plugs shall be suitable to withstand test pressures.

Pipe laying lengths shall not exceed twenty (20) feet in length. Shorter lengths will be required if the CONTRACTOR experiences difficulty in maintaining proper pipe alignment.

A suitable designed water stop shall be utilized with PVC pipe at each manhole connection.

All PVC pipe shall be stored at the project site in strict accordance with the manufacturer's recommendations and at all times prior to actual installation of the pipe the CONTRACTOR shall be responsible for providing uniform support for each length of pipe stored at the site. PVC pipe that has been bowed by the sun shall not be laid until it has straightened and lies flat without restraint.

3.22 **POLYVINYL CHLORIDE (PVC) SEWER FORCE MAIN**

All sewer pipe shall be made from clean, virgin, NSF-approved, Type I, Grade I polyvinyl chloride (PVC), conforming to ASTM resin specification D-1784. All pipe shall meet or exceed minimum requirements of ASTM D-2241 for type 1120 material. SDR classifications as called for on the BID Proposal or minimum SDR-21 wall thickness.

Pipe length shall not exceed twenty one (21) feet unless approved by ENGINEER. Provision must be made for proper transporting, handling and storage of pipe. Pipe and fittings to be assembled with non-toxic lubricant as recommended by manufacturer and approved by ENGINEER. Pipe shall be as manufactured by Johns-Manville, Ethyl Corp., Clow Corp., Certain-Teed, or equal.

Pipe joints shall be the coupling or bell and spigot type utilizing rubber ring compression gasket(s) (ASTM F-477). Provision shall be made for thermal
expansion and contraction to be taken up at the joint.

All pipe shall have a metallic tape or similar device installed in accordance with manufacturer's recommendation. The metallic device shall be Terra Tape or equal and shall be compatible with City location equipment.

Manufacturer shall have pipe tested in accordance with provisions of applicable ASTM Standard. Manufacturer shall furnish ENGINEERS three (3) copies of certified statements to the effect that all items have met or exceeded requirements of the applicable specification. Test certificates will be required unless noted otherwise on drawings and shall cover all pipe used on this project.

All pipe shall be subjected to a rigid inspection after delivery to the site and before being placed in the work. Any item found defective by such field inspection will be rejected and shall be immediately removed from the premises.

Marking shall include the following on each length of pipe: manufacturer's name, nominal size, pressure rating, dimension ratio number, "PVC 1120", ASTM designation number, and NSF seal of approval.

Pipe shall be suitable for use with gray or ductile iron fittings when used with a transition gasket.

3.23 **COMPRESSION COUPLINGS**

When joining different types of pipe together, the CONTRACTOR shall use compression couplings that are resistant to corrosion by soil and sewage and that will provide a permanent watertight joint. The compression coupling shall meet the physical test and joint-leak requirements specified in ASTM C-425 and the bands for attaching sliliner pipe shall be stainless steel conforming to ASTM C-425. Each coupling shall bear the manufacturer's name and an indication of its size.

3.24 **HIGH DENSITY POLYETHYLENE SEWER PIPE**

The pipe and fittings shall be made of high density, high molecular weight polyethylene pipe material meeting the requirements of Type III, Class C, Category 5, Grade P34, as defined in ASTM D-1248 Standard Specification for Polyethylene Plastics Molding and Extrusion Materials. The pipe and fittings shall be homogenous throughout and free from visible cracks, holes, foreign inclusions or other injurious defects. The pipe shall be as uniform as commercially practical in color, opacity, density and other physical properties. The pipe shall be manufactured by the continuous winding of a special profile onto suitably sized mandrels. It shall be produced to constant internal diameters. The pipe shall be Spirolite or equal.

The pipe shall be produced with bell and spigot end construction.

Joining will be accomplished by rubber gasket, or thermal welding, as determined by the design ENGINEER in accordance with the manufacturer's recommendations.

The integral bell and spigot gasketed joint is designed so that when assembled, the elastomeric gasket, contained in a machined groove on the pipe spigot, is compressed radially in the pipe bell to form a positive seal.
Rubber gaskets shall comply in all respects with the physical requirements specified in the non-pressure requirements of ASTM Specification C-443. They shall be molded or produced from an extruded shape approved by the manufacturer and spliced into circular form. The lubricant used for assembly shall have no detrimental effect on the gasket or on the pipe.

The selection and conditioning of pipe samples for testing shall be as established by the ENGINEER. Three (3) specimens of pipe, a minimum of twelve (12) inches long, shall be flattened between parallel plates in a suitable press until the distance between the plates is forty percent (40%) of the outside diameter of the pipe. The rate of loading shall be uniform and such that the compression is completed within two (2) to five (5) minutes.

Remove the load and examine the specimens for splitting, cracking or breaking. There shall be no evidence of splitting, cracking or breaking.

The pipe ring stiffness constant shall be determined utilizing procedures outlined in ASTM D-2412. Test specimens shall be a minimum of two pipe diameters or four (4) feet in length, whichever is less. Ring Stiffness Constant (RSC) values for the pipe can be directly related to the pipe's class designation. When tested, the minimum RSC shall be ninety percent (90%) of the nominal.

All pipe shall be clearly marked to show the pipe size, class and profile number and the production code.

3.25 FLOWABLE FILL

All flowable fill mortar shall be in accordance with the Standard Specifications for Road and Bridge Construction except as modified herein.

<table>
<thead>
<tr>
<th>MATERIAL</th>
<th>SUBSECTION</th>
</tr>
</thead>
<tbody>
<tr>
<td>Portland Cement, Type I</td>
<td>901.01</td>
</tr>
<tr>
<td>Fly Ash, Class C or Class F</td>
<td>AASHTO M 295</td>
</tr>
<tr>
<td>Water</td>
<td>918.01</td>
</tr>
<tr>
<td>Chemical Additives</td>
<td>918.09</td>
</tr>
</tbody>
</table>

Fine Aggregate shall conform to the requirements Subsection 903.01. Fine aggregate for Concrete except that the gradation shall be as follows:

<table>
<thead>
<tr>
<th>SIEVE SIZE</th>
<th>PERCENT PASSING</th>
</tr>
</thead>
<tbody>
<tr>
<td>¾ - inch</td>
<td>100</td>
</tr>
<tr>
<td>No. 200</td>
<td>0-10</td>
</tr>
</tbody>
</table>

Flowable fill mortar shall be proportioned as follows:

<table>
<thead>
<tr>
<th>MATERIAL</th>
<th>PER CUBIC YARD</th>
</tr>
</thead>
<tbody>
<tr>
<td>Portland Cement, Type I</td>
<td>100 lbs (Maximum)</td>
</tr>
<tr>
<td>Fly Ash, Class C or Class F</td>
<td>250 lbs (Minimum)</td>
</tr>
<tr>
<td>Fine Aggregate</td>
<td>2800 lbs</td>
</tr>
<tr>
<td>Water</td>
<td>60 gal (Approximate)</td>
</tr>
</tbody>
</table>

The above proportions may be adjusted by the Engineer to obtain the consistency required for satisfactory flow. Consistency shall be determined as follows:

Place an open-ended cylinder (pipe) three inches in diameter by six inches in height in an upright position on a smooth, level surface. Fill the cylinder with a representative sample of the flowable fill mortar proposed for use. Remove the
cylinder by lifting it straight up thus allowing the sample to diffuse on the smooth, level surface. The flowable fill mortar should diffuse into a circular shape having an approximate diameter of not less than eight inches.

3.26 **Flygt Submersible Pumps**

**A. REQUIREMENTS**

Furnish and install submersible non-clog wastewater pump(s). Each pump shall be equipped with an HP submersible electric motor, connected for operation on volts, Phase, 60 hertz, wire service, with feet of submersible cable (SUBCAB) suitable for submersible pump applications. The power cable shall be sized according to NEC and ICEA standards and also meet with P-MSHA Approval.

**B. PUMP DESIGN CONFIGURATION (Dry pit installation)**

Pump shall be capable of operating in a continuous non-submerged condition in horizontal (NZ) or vertical (NT) (select one) position in a dry pit installation and permanently connected to inlet and outlet pipes. Pump shall be of submersible construction and will continue to operate satisfactorily should the dry pit be subjected to flooding.

**C. PUMP DESIGN CONFIGURATION (Wet pit installation)**

The pump shall be supplied with a mating cast iron inch discharge connection and be capable of delivering GPM at FT. TDH. The pump(s) shall be automatically and firmly connected to the discharge connection, guided by no less than two guide bars extending from the top of the station to the discharge connection. There shall be no need for personnel to enter the wet-well. Sealing of the pumping unit to the discharge connection shall be accomplished by a machined metal-to-metal watertight contact. Sealing of the discharge interface with a diaphragm, O-ring or profile gasket will not be acceptable. No portion of the pump shall bear directly on the sump floor. Each pump shall be fitted with feet of lifting chain or stainless steel cable. The working load of the lifting system shall be 50% greater than the pump unit weight.

**D. PUMP CONSTRUCTION**

Major pump components shall be of grey cast iron, ASTM A-48, Class 35B, with smooth surfaces devoid of blowholes or other irregularities. All exposed nuts or bolts shall be of stainless steel construction. All metal surfaces coming into contact with the pump age, other than stainless steel or brass, shall be protected by a factory applied spray coating of acrylic dispersion zinc phosphate primer with a polyester resin paint finish on the exterior of the pump. Sealing design shall incorporate **metal-to-metal contact** between machined surfaces. Critical mating surfaces where watertight sealing is required shall be machined and fitted with Nitrile or Viton rubber O-rings. Fittings will be the result of controlled compression of rubber O-rings in two planes and O-ring contact of four sides without the requirement of a specific torque limit.

Rectangular cross-sectioned gaskets requiring specific torque limits to achieve compression shall not be considered as adequate or equal. No secondary sealing compounds, elliptical O-rings, grease or other devices shall be used.
E. COOLING SYSTEM

(Non-cooling Jacket Equipped)

Each pump motor shall be sufficiently cooled by the surrounding environment or by submergence in the pumped media.

(Cooling Jacket Equipped)

Each unit shall be provided with an integral motor cooling system. A motor cooling jacket shall encircle the stator housing, providing for dissipation of motor heat regardless of the type of pump installation. An impeller, integral to the cooling system and driven by the pump shaft, shall provide the necessary circulation of the cooling liquid through the jacket. The cooling liquid shall pass about the stator housing in the closed loop system in turbulent flow providing for superior heat transfer. The cooling system shall have one fill port and one drain port integral to the cooling jacket. The cooling system shall provide for continuous pump operation in liquid or ambient temperatures of up to 104°F. (40°C.). Operational restrictions at temperatures below 104°F are not acceptable. Fans, blowers or auxiliary cooling systems that are mounted external to the pump motor are not acceptable.

F. CABLE ENTRY SEAL

The cable entry seal design shall preclude specific torque requirements to insure a watertight and submersible seal. The cable entry shall consist of dual cylindrical elastomer grommets, flanked by washers, all having a close tolerance fit against the cable outside diameter and the entry inside diameter. The grommets shall be compressed by the cable entry unit, thus providing a strain relief function. The assembly shall provide ease of changing the cable when necessary using the same entry seal. The cable entry junction chamber and motor shall be sealed from each other, which shall isolate the stator housing from foreign material gaining access through the pump top. Epoxies, silicones, or other secondary sealing systems shall not be considered equal.

G. MOTOR

The pump motor shall be a 3 phase NEMA B design, induction type with a squirrel cage rotor, shell type design, housed in an air filled, watertight chamber. The stator windings shall insulated with moisture resistant Class H insulation rated for 180°C (356°F). The stator shall be insulated by the trickle impregnation method using Class H monomer-free polyester resin resulting in a winding fill factor of at least 95%. The motor shall be inverter duty rated in accordance with NEMA MG1, Part 31. The stator shall be heat-shrink fitted into the cast iron stator housing. The use of multiple step dip and bake-type stator insulation process is not acceptable. The use of pins, bolts, screws or other fastening devices used to locate or hold the stator and that penetrate the stator housing are not acceptable. The motor shall be designed for continuous duty while handling pumped media of up to 104°F. The motor shall be capable of no less than 15 evenly spaced starts per hour. The rotor bars and short circuit rings shall be made of aluminum. Three thermal switches shall be embedded in the stator end coils, one per phase winding, to monitor the stator temperature. These thermal switches shall be used in conjunction with and supplemental to external motor overload protection and shall be connected to the motor control panel. The junction chamber shall be sealed off from the stator housing and shall contain a terminal board for connection of power and pilot sensor cables using threaded compression type terminals. The use of wire nuts or crimp-type connectors is not acceptable. The motor and the pump shall be produced by the same manufacturer.

The motor service factor (combined effect of voltage, frequency and specific gravity) shall be 1.15. The motor shall have a voltage tolerance of +/- 10%. The motor shall be designed for continuous operation in up to a 40°C. ambient and shall have a NEMA
Class B maximum operating temperature rise of 80° C. A motor performance chart shall be provided upon request exhibiting curves for motor torque, current, power factor, input/output kW and efficiency. The chart shall also include data on motor starting and no-load characteristics.

Motor horsepower shall be sufficient so that the pump is non-overloading throughout its entire performance curve, from shut-off to run-out. The motor and cable shall be capable of continuous submergence underwater without loss of watertight integrity to a depth of 65 feet or greater.

H. BEARINGS

The integral pump/motor shaft shall rotate on two bearings. The motor bearings shall be sealed and permanently grease lubricated with high temperature grease. The upper motor bearing shall be a single ball type bearing to handle radial loads. The lower bearing shall be a two-row angular contact ball bearing to handle the thrust and radial forces. The minimum L10 bearing life shall be 50,000 hours at any usable portion of the pump curve.

I. MECHANICAL SEALS

Each pump shall be provided with a positively driven dual, tandem mechanical shaft seal system consisting of two seal sets, each having an independent spring. The lower primary seal, located between the pump and seal chamber, shall contain one stationary and one positively driven rotating corrosion resistant tungsten-carbide ring. The upper secondary seal, located between the seal chamber and the seal inspection chamber, shall contain one stationary and one positively driven rotating corrosion resistant tungsten-carbide seal ring. All seal rings shall be individual solid sintered rings. Each seal interface shall be held in place by its own spring system. The seals shall not depend upon direction of rotation for sealing. Mounting of the lower seal on the impeller hub is not acceptable. Shaft seals without positively driven rotating members or conventional double mechanical seals containing either a common single or double spring acting between the upper and lower seal faces are not acceptable. The seal springs shall be isolated from the pumped media to prevent materials from packing around them, limiting their performance.

Each pump shall be provided with a lubricant chamber for the shaft sealing system. The lubricant chamber shall be designed to prevent overfilling and shall provide capacity for lubricant expansion. The seal lubricant chamber shall have one drain and one inspection plug that are accessible from the exterior of the motor unit. The seal system shall not rely upon the pumped media for lubrication.

The area about the exterior of the lower mechanical seal in the cast iron housing shall have cast in an integral concentric spiral groove. This groove shall protect the seals by causing abrasive particulate entering the seal cavity to be forced out away from the seal due to centrifugal action.

A separate seal leakage chamber shall be provided so that any leakage that may occur past the upper, secondary mechanical seal will be captured prior to entry into the motor stator housing. Such seal leakage shall not contaminate the motor lower bearing. The leakage chamber shall be equipped with a float type switch that will signal if the chamber should reach 50% capacity.

J. PUMP SHAFT

The pump and motor shaft shall be a single piece unit. The pump shaft is an extension of the motor shaft. Shafts using mechanical couplings shall not be acceptable. The shaft shall be stainless steel — ASTM A479 543100-T. Shaft sleeves will not be acceptable.
K. IMPELLERS

(Cast Iron - .181 and .091 versions)
The impeller shall be of gray cast iron, ASTM A-48 Class 35B, dynamically balanced, semi-open, multi-vane, back swept, screw-shaped, non-clog design. The impeller leading edges shall be mechanically self-cleaned automatically upon each rotation as they pass across a spiral groove located on the volute suction. The screw-shaped leading edges of the impeller shall be hardened to Rc 45 and shall be capable of handling solids, fibrous materials, heavy sludge and other matter normally found in wastewater. The screw shape of the impeller inlet shall provide an inducing effect for the handling of up to 6% sludge and rag-laden wastewater. The impeller to volute clearance shall be readily adjustable by the means of a single trim screw. The Impeller shall be locked to the shaft and held by an impeller bolt.

(High-chrome iron - .185 and .095 versions)
The impeller shall be of ASTM A 532 (Alloy III A), 25% chrome cast iron, dynamically balanced, semi-open, multi-vane, back swept, screw-shaped, non-clog design. The impeller leading edges shall be mechanically self-cleaned automatically upon each rotation as they pass across a spiral groove located on the volute suction. The screw-shaped leading edges of the impeller shall be capable of handling solids, fibrous materials, heavy sludge and other matter normally found in wastewater. The screw shape of the impeller inlet shall provide an inducing effect for the handling of up to 6% sludge and rag-laden wastewater. The impeller to volute clearance shall be readily adjustable by the means of a single trim screw. The Impeller shall be locked to the shaft and held by an impeller bolt.

L. VOLUTE/SUCTION COVER

The pump volute shall be a single piece gray cast iron, ASTM A-48, Class 35B, non-concentric design with smooth passages of sufficient size to pass any solids that may enter the impeller. Minimum inlet and discharge size shall be as specified. The volute shall have a replaceable volute insert ring containing spiral-shaped, sharp-edged groove(s). The spiral groove(s) shall provide the relief path and sharp edge(s) across which each impeller vane leading edge shall cross during rotation so to remain unobstructed. The internal volute bottom shall provide effective sealing between the multi-vane semi-open impeller and the volute. The insert ring shall be cast of (ASTM A-48 Class 35B cast iron or ASTM A 532 (Alloy III A), 25% chrome cast iron)

M PROTECTION

Each pump motor stator shall incorporate three thermal switches, one per stator phase winding and be connected in series, to monitor the temperature of the motor. Should the thermal switches open, the motor shall stop and activate an alarm. A float switch shall be installed in the seal leakage chamber and will activate if leakage into the chamber reaches 50% chamber capacity, signaling the need to schedule an inspection.

The thermal switches and float switch shall be connected to a Mini CAS control and status-monitoring unit. The Mini CAS unit shall be designed to be mounted in the pump control panel.

N. System Controls

The control system shall be designed to operate the required number of pumps specified on the drawing at the power characteristics shown on the plans.

The control function shall provide for the operation of the pumps under normal conditions, and shall alternate the pumps on each pump down cycle to equalize the run time. In the event the incoming flow exceeds the pumping capacity of the lead pump,
subsequent pumps shall automatically start to handle the increased flow. As the flow decreases, the pumps shall cut off at the elevations as shown on the plans.

The control shall function as described below. The equipment listed below is a guide and does not relieve the supplier from supplying a system that will function as required.

O. **Mechanical:**

The enclosure shall be a NEMA 4X Stainless steel enclosure. The enclosure shall be a wall mount type with a minimum depth of 10” sized to adequately house all the components. Enclosures larger than 60" high x 36" wide shall be provided with 12” high leg stands. The enclosure door gaskets shall be rubber composition with a retainer or seamless foamed in place to assure a positive weatherproof seal. The gasket material shall not retain memory. The door shall open a minimum of 180 degrees.

A polished aluminum dead front inner door shall be mounted on a continuous aircraft type hinge and shall contain cutouts for mounted equipment and provide protection of personnel from live internal wiring. Cutouts for breaker handles shall be provided to allow operation of breakers without entering the compartment. All control switches, indicator pilot lights, elapsed time meters, and other operational devices shall be mounted on the external surface of the dead front. The dead front shall open a minimum of 150 degrees to allow access to equipment for maintenance. A 3/4” break shall be formed around the perimeter of the dead front to provide rigidity.

The back plate shall be manufactured of 12 gauge sheet steel and be finished with a primer coat and two [2] coats of baked on white enamel. All hardware mounted to the subpanel shall be accomplished with machine thread tapped holes. Sheet metal screws are not acceptable. All devices shall be permanently identified using engraved nameplates. Use of DYMO type labels is not acceptable.

P. **Electrical:**

The panel power distribution shall include all necessary components and be completely wired with tinned, stranded copper conductors rated at 90 degrees c. All conductor terminations shall be as recommended by the device manufacturer.

All circuit breakers shall be heavy-duty thermal magnetic or motor circuit protectors similar and equal to SQUARE D type FAL. Each motor breaker shall be adequately sized to meet the pump motor operating characteristics and shall have a minimum of 10,000 amps interrupting capacity for 230 VAC and 18,000 amps at 480 VAC. The control circuit shall individually be controlled by a heavy-duty breaker.

Circuit breakers shall be indicating type, providing "on-off-trip" positions of the operating handle. When the breaker is tripped automatically, the handle shall assume a middle position indicating "trip".

Thermal magnetic motor breakers shall be quick-make and quick-break on manual and automatic operation and have inverse time characteristics secured through the use of bimetallic tripping elements supplemented by a magnetic trip.

Breakers shall be designed so that an overload on one pole automatically trips and opens all legs. Field installed handle ties shall not be acceptable. Motor starters shall be open frame, across the line NEMA rated with individual overload protection in each leg. Motor starter contact and coil shall be replaceable from the front of the starter without removing from its mounted position. Overload heaters shall be block type, utilizing melting alloy spindles, and shall have visual trip indication. Overload
shall be sized for the full load amperage draw of the pumps. Definite purpose
contactors, fractional size starters and IEC contactor relays shall not be acceptable.

Control transformers shall be provided to provide the 120 VAC and/or 24 VAC for
control circuits when required. Transformers shall be fused on the primary and
secondary circuits. The secondary windings shall be grounded.

A lightning-transient protector with tell-tale warning lights on each phase to indicate
loss of protection on the individual phases shall be provided. The device shall be solid
state with a response time of less then 5 nanoseconds with withstanding surge
capacity of 6500 amperes. Unit shall be instant recovery, long life and have no
holdover currents

The Phase Monitor shall be a 12 pin, plug in style unit. The Phase Monitor shall
monitor Under Voltage, Phase Reversal, Loss of Power and Phase Imbalance.
The motor starter circuits shall be de-energized upon sensing of any of the
faults and shall automatically restore service upon return to normal power. The
Phase Monitor shall be available to monitor Over Voltage as an option. The
output relay shall be DPDT rated at 10A at 240 VAC. The Phase Monitor shall
be model 001-2301212, or model 001-480-1212 as manufactured by Motor

Q. Alarm System:

The alarm light shall be a weatherproof, shatterproof, red light
fixture with a 40-watt bulb to indicate alarm conditions. The alarm light shall be
turned on by the high-level alarm and flash until the condition has been corrected.
An open contact shall be provided for remote monitoring.

The alarm horn shall be mounted on the exterior of the cabinet.
The alarm horn shall provide a signal of not less than 90db at 10 feet. The alarm
horn shall not degrade the listing of the enclosure. An alarm silence switch shall
deactivate the alarm horn; however, the alarm light will flash until the alarm
condition ceases to exist. At that time, the alarm reset function will reset for
normal operation.

R. Level Control System:

(A) 24V FLOAT RELAY SYSTEM:

A 24-volt ac control system shall be provided for the float control system. The
system shall provide for the automatic and manual control and alternation of the pumps
to maintain a pumped down condition of the wet well. Float regulators adjusted to
the level shown on the plans shall sense levels. A float regulator shall control each
pump and when tilted shall turn the pump on. The pump (s) shall remain "on" until a
common "off" level is reached. In the event the "off" float regulator fails, the system
shall sense the failure and switch the "off" level to the second float regulator. The
system shall provide indication for the regulators and indicate a failure of the "off"
unit.

Control systems contingent on the "off" float regulator supplying control power to
the other units is not acceptable.

A three-position HOA switch shall be provided for each pump. The switch shall be
NEMA 4x rated with 10 amp contacts except when provided on a dedicated
controller it. A position indicating legend plate shall be provided. The HOA switches
shall be mounted on the inner dead front door unless provided in the controller units.
A green run pilot indicator shall be mounted on the dead front door. Level indicator lights or indicators shall be provided.

An elapsed time meter shall be mounted on the dead front door. The meter shall operate on 120 VAC, shall indicate in hours [6 digits] and tenths and shall be non-resettable.

The alternator shall be a plug in, solid-state unit with lead-lag auto selector and test switches except when provided in a dedicated control device. The unit shall operate on 120 vac and provide DPDT ten amp rated contacts. Two LEDs shall indicate the next position to run as lead pump.

A thermal heater and thermostat shall be installed to maintain the internal temperature of the enclosure above the dew point.

Control wiring shall be copper, tinned, UL1015, 18ga. minimum.

(B) MINI CAS

One Mini-Cas 12O unit shall be supplied for each pump to monitor the pump for over-temp and leakage. The unit shall have an 11 pin, round base to mate with a standard 11-pin socket. The unit shall also be flanged in order to allow dead front door mounting with use of 11 pin reverse socket, Omron part number P3GA-11. The unit is to be able to be powered by 24VAC, 24VDC, or 12OVAC, and to contain LED indication for power on, over-temp, and leakage conditions. The unit shall contain an over-temp reset bush-button to reset the unit after the fault has cleared, as well as a selector switch that allows the selection of manual or auto reset. The sensor input circuitry is to contain both hardware and software filters for noise immunity, as well as sensor input short circuit protection. The Mini-Cas 12O unit shall be model 14-407129, as supplied by Flygt Corporation.

(C) FLOAT SWITCH

A mechanical float switch shall be supplied for level control and be suspended at the desired height from its own cable. The float switch case shall be made of polypropylene and the cable is sheathed with a special PVC compound. The float switch cables shall be supplied with 40' of cable.

S. Manufacturer:

A final as built drawing encapsulated in mylar shall be attached to the inside of the front door. Schematics shall be done in ladder logic with wire numbers and line numbers. Real time cross-referencing of relay contact to line numbers shall be given as well as written description of component function on each circuit of the drawings. From/to wire and termination reports shall be shown on the as built drawings. Drawings shall be available in HTML format. Terminal strip layouts shall be provided for ease of connecting external devices.

All component parts in the control panel shall be permanently identified with engraved legend plates as designated on the drawings. A list of all legends shall be available in Excel format and attached with the schematics on the panel door.

All equipment shall be tested to the operational requirements. Each control function shall be activated to check for proper indication.

All equipment shall be guaranteed for a period of one year from the date of installation. The
guarantee is effective against all defects in workmanship and/or defective component. The warranty is limited to replacement of or repair of the defective equipment.

The manufacturer shall be a UL508 shop and provide evidence on the end product.

3.27 Remote Telemetry Unit

A. SUBMITTALS

General

All Shop Drawings and O&M Manuals shall be furnished in hard copy and in Adobe PDF electronic format on compact disk

Radio Survey

1. Has been previously completed by others. A copy of the radio survey results will be submitted to the Systems Integrator.
2. To be reviewed by Systems Integrator to confirm that radio communication links will be reliable.
3. Any required towers, poles, or masts are not included in this Specification Section.

Hardware

1. Submit full details, shop drawings, wiring diagrams, catalog cuts and such descriptive literature and documentation as may be required to demonstrate conformance to these specifications. These submittals shall clearly identify specific items and options proposed to be provided. In addition, these submittals shall clearly identify installed spares and other provisions for future work (e.g. reserved panel space, unused components, wiring, terminals, etc.)

2. Legends and Abbreviations Lists: As part of the first Design Related Submittals for each subsection, submit a complete definition of symbols and abbreviations used on this project. For example: engineering units, flow streams, instruments, structures and other process items used in nameplates, legends, data sheets, point descriptions, CRT displays, alarm/status logs and reports. The same abbreviations shall be used for the subsections. Submit updated versions with subsequent submittals and a final version with the O&M Manuals.

3. Cross Referencing of Components: The Table of Contents of the Submittal and O&M manual shall provide for each item: Component Data Sheet No., Specification Section Reference No., Manufacturer, Model No., Description, Instrument I.D., Tag No. and Calibration Range (or field devices). Furthermore, each item's individual Component Data Sheet shall reference the Specification Section Reference No., the Supplier's Drawing No. that has the Bill of Material for the Panel in which the component is mounted, the Item number for the component in said Bill of Material, Instrument I.D., Tag No. and Calibration Range (for field devices). This information will be in addition to the items specified in sections entitled Component Data Sheets and Bill-of-Materials. The Table of Contents shall be sorted by Specification Section in ascending order.

4. Notification of Minor Deviations from Specification: Any deviation from specifications shall be noted at the front of the submittal in a separate
section labeled Items of Interest, Comments, and Requests for Clarification. This section shall detail any minor deviations from specifications herein. Each Item of Interest, Comment, or Request for Clarification shall be numbered and the specification section in question shall be referenced. These items shall be sorted by Specification Section in ascending order. The intent of this section is to make Engineer aware of any minor change from specification because of outdated equipment, differences in panel manufacturing from supplier to supplier, etc. Approval of such minor deviations will be at the sole discretion of the Engineer. No change in contract price will be allowed if the minor deviations are acceptable.

Software

1. All software submittals shall be in accordance with the requirements of the associated subsection sections. Their sequences of development shall also be in accordance with the associated requirements stated herein.

B. TESTING

General

1. All elements of the SCADA System, both hardware and software, shall be tested to demonstrate that the total system satisfies all of the requirements of this specification.

2. Each test shall be in the cause and effect format. The person conducting the test shall initiate an input (cause) and upon the system's or subsection's producing the correct result (effect), the specific test requirement will have been satisfied.

3. All tests shall be conducted in accordance with, and documented on, prior engineer approved procedures, forms and checklists. Each specific test to be performed shall be described and a space provided after it for sign off by the appropriate party after its satisfactory completion.

4. Copies of these sign off test procedures, forms and checklists will constitute the required test documentation.

5. Provide all special testing materials and equipment. Wherever possible, perform tests using actual process variables, equipment and data. Where it is not practical to test with real process variables, equipment and data, provide suitable means of simulation. Define these simulation techniques in the test procedures.

C. O&M MANUALS

A. General

1. Provide up to (10) complete sets of separately bound Operation and Maintenance (O&M) Manuals for each subsection of the SCADA System. O&M Manuals shall also be furnished in Adobe PDF electronic format on compact disk.

2. These manuals shall include operating and maintenance information for all subsections (and their components) provided in this section. The O&M information shall be in sufficient detail to allow the operation, removal, installation, adjustment, calibration and maintenance of
each component provided under this section to the printed circuit board level.

3. Each manual shall include a legend and abbreviation list.

4. Each set of manuals for each subsection shall be assembled in one or more 3-ring binders; each with a title page, table of contents, and heavy section dividers with labeled index tabs. Where more than one binder is required, the binders shall be labeled "Volume 1", "Volume 21", etc. The table of contents shall encompass the entire set of O&M Manuals, shall list the contents of each volume and shall appear in each binder.

5. Operational Readiness Test Component and I/O Module Calibration Sheets shall be included as part of the Analog Subsection O&M Manual sets.

D. ON-SITE SERVICES

A. General

1. The Contractor shall require the SCADA System supplier to provide experienced personnel and management on-site to coordinate the complete SCADA System which shall include the following as a minimum:

   a. Instrument calibration and adjustment
   b. All on-site testing
   c. Owner training
   d. Start-up assistance

B. Start-up and Testing Team

1. The Contractor shall require the SCADA System supplier to provide, on-site, a team of experienced engineering, technician and software/configuring personnel during the total period required to:

   a. Thoroughly check the installation, termination and adjustment of all of the subsections and their components.
   b. Perform and complete all on-site tests.
   c. Provide start-up assistance to the Contractor and Owner.

E. PUMP STATION REMOTE TERMINAL UNIT

Panel hardware

1. Bristol Controwave Micro
2. NEMA 4 Enclosure with sub-pan
3. Door operated switch for enclosure
4. MDS 9810 Radio
5. 24 VDC Power Supply
6. 700 VA UPS
7. Cable & Polyphaser
8. Yagi 12db Gain Antenna
9. Heater with Thermostat & Fan
10. Duplex Receptacle
11. Fuses and breakers as required
12. Name Plates
13. A/C Surge suppressor
14. Surge suppressor for each outside mounted analog signal
15. Weidmuller terminal blocks as needed

**Discrete I/O (supply 20% spare)**

1. Pump 1 Run Status
2. Pump 2 Run Status
3. Pump 3 Run Status
4. Pump 1 Alarm
5. Pump 2 Alarm
6. Pump 3 Alarm
7. Utility Power OK
8. Utility Power Failure
9. Generator Failure
10. Sump High Water Level Switch
11. Sump High High Water Level Switch
12. Door Switch Status
13. Grinder 1 Run Status
14. Grinder 2 Run Status
15. Bar Screen 1 Run Status
16. Bar Screen 2 Run Status

**Analog I/O (supply 20% spare)**

1. Wet well Level
2. Pump 1 Speed (0-100%)
3. Pump 2 Speed (0-100%)
4. Pump 3 Speed (0-100%)
5. Pump Station Flow (where required)

**F. INSTRUMENTS**

**A. Magnetic Flow Meter with remote mounted transmitter and display (where required)**

1. Magmeter is to be Danfoss with MAG 6000 remote display transmitter.
2. ANSI Class 150 lb flanges
3. 0.25% accuracy with improved accuracy at low flow
4. Hard elastomer lined bore
5. Soft elastomer facing
6. Stainless steel measuring and grounding electrodes
7. Conforms to OIML R49 and ISO 4064 custody transfer standards
8. Operating temperature from 23 — 195 °F
9. Confirms to EEC directives: PED, 97/23/EC pressure directive for EN1092-1 flanges
10. UL approved

**B. Ultrasonic level Transducer & Transmitter**

1. Each ultrasonic level transmitter shall be a microprocessor-based electronic unit consisting of a sensor assembly, a signal converter/transmitter, and an interconnecting cable.
2. The sensor shall be encapsulated in a chemical- and corrosion-resistant material such as kynar or CPVC, and shall be suitable for operation over a temperature range of -20 to 150 °F, with a relative humidity of 10 to 100 percent.
3. Sensors shall be compatible with the process media being measured. The sensor shall be mounted directly over the Process and shall measure the fluid level by means of reflected high frequency sound waves.

4. Sensors mounted in areas subject to freezing condensation shall be protected against icing with special transducers or heaters. Sensors mounted in direct sunlight shall be provided with sunshades.

5. The supplier shall coordinate the sensor mounting requirements and shall furnish drawings, complete with dimensions and elevations, to ensure a proper and satisfactory installation. General installation requirements are indicated on the drawings.

6. The ultrasonic level transmitter shall have automatic compensation for changes in air temperature at the sensor location. If separate temperature sensing probes are provided, they shall be mounted with or adjacent to the ultrasonic sensor, as recommended by the manufacturer.

7. The transmitter shall have a four-digit LCD display scaled to read in engineering units of level. Digit height shall be approximately 0.5 inch. The transmitter shall be designed to ignore momentary level spikes, false targets, or momentary loss-of-echo. A loss-of-echo condition shall be indicated on the transmitter unit and shall be available as an alarm contact output.

8. The transmitter output shall be an isolated 4-20 mA dc signal linearly proportional to level.

9. Calibration parameters shall be stored in nonvolatile EEPROM memory.

10. Accuracy of the transmitted signal shall be 1.0 % of the level range.

11. A sufficient length of sensor-to-transmitter signal cable shall be furnished.

12. The signal converter electronics shall be housed in a NEMA Type 4 enclosure suitable for wall or pipe stand mounting and for operating temperatures of -15 to 125 °F., and a relative humidity of 10 to 100 percent. A thermostatically controlled strip heater shall be provided in the signal converter enclosure. The signal converter shall be powered from 120 volts ac, 60 Hz.

13. The ultrasonic level transmitter shall be a Pulsar. The instruments shall be suitable for the locations in which they will be installed so that they will not be affected by adjacent walls, etc.

14. Ultrasonic Level Transmitter shall be provided for the wet well.

C. Level Switch (Float Type)

1. Type: Submersible coated 316 stainless steel, polypropylene, or polyethylene body; non mercury switch contact rated 4 amps at 120 VAC; normally open, normally closed, or Form C (N.O and N.C.) contact configuration as indicated. Mercury float switches are unacceptable.
2. Cable. Minimum 18 gauge, 300 volt (minimum) rated; heavy-duty type SOW or equivalent. Provide sufficient length for mounting.

3. Junction Box: Provide NEMA 4X stainless steel junction box, mounted near the switch, for terminating vendor supplied cable and discrete control wiring to control panel.

4. Switch Mounting: All mounting components shall be aluminum or stainless steel. The Electrical Contractor shall provide 3/4 or 1 inch Schedule 40, 316 stainless steel pipe for mounting. The pipe shall extend from two feet above the highest switch setting (up to the top of the vessel) to two feet below the lowest level setting (down to the vessel bottom) for the vessel or well and allow for adjustment of the switch or along the length of the pipe. The method for fixing the float to the pipe shall be easily adjustable and shall provide protection and strain relief for the float switch cable. Provide a minimum of two mounting brackets for fixing the pipe to the vessel wall while maintaining appropriate standoff distance. The System Manufacturer shall ensure mounting is in accordance with the manufacturer’s recommendations.

(Note: installation and mounting of all instruments is the responsibility of the General Contractor.)

G. CONTROL PANELS, CABINETS AND COMPONENTS

A. General Requirements

1. Furnish all labor, materials, equipment, and incidentals required to fabricate and startup, complete and ready for operation. The panels listed in this section to be supplied by the System Integrator.

2. All control panels furnished under this Section shall carry a UL label which certifies the control panel meets the requirements of UL-508A (latest version).

3. The Control systems shall be Underwriters Laboratories UL508A approved and all components shall be NEMA rated. IEC components will not be accepted. All control panels shall be assembled by a UL approved shop and labeled to that effect.

4. All electrical work shall be in accordance with the National Electrical Code (NEC), latest revision.

5. Submit full details, shop drawings, wiring diagrams, catalog cuts and such descriptive literature and documentation as may be required to demonstrate conformance to these specifications. These submittals shall clearly identify specific items and options proposed to be provided. In addition, these submittals shall clearly identify installed spares and other provisions for future work (e.g. reserved panel space, unused components, wiring, terminals, etc.)

6. All Shop Drawings and O&M Manuals shall be furnished in hard copy and in Adobe PDF electronic format on compact disk.
SECTION 4 - EXCAVATION & BACKFILL

4.01  GENERAL

The CONTRACTOR shall perform all required excavation and backfilling incidental to the installation of force mains, sewers, manholes, and other appurtenances under this contract. Excavation shall be carried to the depths indicated on the drawings or as necessary to permit the installation of pipe, bedding, structures of appurtenances. Care shall be taken to provide a firm, undisturbed, uniform surface in the bottoms of trenches and excavations for structures. Where the excavation exceeds the required depth, the CONTRACTOR shall bring the excavation to proper grade through the use of an approved incompressible backfill material (generally crushed stone or fill concrete, depending upon the nature of the facility to be placed thereon). In the event unstable soil conditions are encountered at the bottom of the excavation, the ENGINEER may direct the CONTRACTOR to continue the excavation the firm soil or to provide pilings or other suitable special foundations.

The CONTRACTOR shall take such precautions as may be necessary to avoid endangering personnel, pavement, adjacent utilities or structures through cave-ins, slides, settlement or other soil disturbance resulting from his operations.

Backfilling shall be carried out as expeditiously as possible, but shall not be undertaken until the ENGINEER has been given the opportunity to inspect the work. The CONTRACTOR must carry out all backfilling operations with due regard for: the protection of pipes, structures and appurtenances; the use of prescribed backfill materials; and procedures to obtain the desired degree of compaction.

The CONTRACTOR shall be responsible for storage of excavated material, disposal of surplus excavated material, trench dewatering and other operations incidental to excavation and backfilling operations.

4.02  CLASSIFICATION OF EXCAVATION

Excavation shall be classified only as earth excavation and solid rock excavation. Solid rock excavation shall consist of the removal of all rock larger than nine (9) cu. ft. in volume that cannot be removed by normal trenching or excavating equipment. Material that can be loosened or separated with a pick or that can be excavated with a trencher or backhoe will not be classified as solid rock excavation.

4.03  TRENCH EXCAVATION

Trenches shall be neatly excavated to the alignment and depth required for the proper installation of pipe, bedding material and appurtenances. Trenches shall be opened up far enough ahead of pipe laying to reveal obstructions, but in general shall not include more than 300 feet of continuous open trench at any time. the CONTRACTOR will be required to follow up trenching operations promptly with pipe laying, backfill and clean-up, and in the event of failure to do so, may be prohibited from opening additional trench until such work is completed.
The CONTRACTOR shall plan his operations so as to cause a minimum of inconvenience to property owners and to traffic. No road, street or alley may be closed unless absolutely necessary, and then only if the following conditions are met:

1. Permit is secured from appropriate State, County or Municipal authorities having jurisdiction.
2. Fire and Police Departments are notified before road is closed.
3. Suitable detours are provided and are clearly marked.

No driveways shall be cut or blocked without first notifying the occupant of the property. Every effort shall be made to schedule the blocking of drives to suit the occupants convenience, and except in cases of emergency, drives shall not be blocked for a period of more than 8 hours.

The CONTRACTOR shall furnish and maintain barricades, signs, flashing lights, and other warning devices as necessary for the protection of public safety. Flagmen shall be provided as required on heavily traveled streets to avoid traffic jams or accidents.

Trench width shall be held to a minimum consistent with proper working space for assembly of pipe. Maximum trench width up to a point one foot (1') above top of pipe shall be limited to the outside pipe diameter plus eighteen inches (18"). Boulders, large stone, shale and rock shall be removed to provide clearance of six inches (6") below and on each side of the pipe.

Trench walls shall be kept as nearly vertical as possible with due consideration to soil conditions encountered and when necessary, sheeting or bracing shall be provided to protect life and property.

Where unstable soil conditions are encountered at the trench bottom, the CONTRACTOR shall remove such additional material as may be directed by the ENGINEER and replace the excavated material with approved backfill.

The CONTRACTOR shall excavate by hand wherever necessary to protect existing structures or utilities from damage or to prevent over excavation in the trench subgrade.

Excavated material shall be stored safely away from the edge of trench and in such a way as to avoid encroachment on private property.

The trench shall be excavated to sufficient depth to permit a minimum of thirty inches (30") of cover to be maintained over the top of sewer force mains. The bottom of the trenches must be shaped by hand and bell holes must be dug so that the full length of pipe is resting on trench bottom. Blocking shall not be used and neither shall the pipe be laid on a trench bottom that has not been leveled to provide support throughout the full length of the pipe.

The CONTRACTOR'S attention is call to the fact that the thirty inch (30") depth of cover is a minimum and may be exceeded in instances where obstructions are encountered in trenching operations. The CONTRACTOR will be permitted to lay the sewer force main above the obstruction only if the minimum cover required can be obtained while providing a cushion at least six inches (6") thick between the bottom of the pipe and the top of the obstruction. Where this minimum cover and the required clearance cannot be obtained the CONTRACTOR will be required to
lay the pipe under the obstruction and will receive no additional compensation for
the additional depth of trench required for constructing the line in this manner. The
CONTRACTOR will also be required to gradually increase the depth of trench
when approaching cuts, creek banks, or other changes in grade in order to avoid
the use of fittings, wherever it is practical to do so.

Trenches for sewers shall be carefully excavated to maintain the desired grade and
alignment. Depth of finished trench shall be adequate to accommodate the
bedding as specified in Section 6.

4.04 EXCAVATION FOR STRUCTURES

Excavation for manholes, junction boxes, piers or other structures shall be only as
large as may be required for the structure and for working room around the
structure. In earth, excavation shall generally extend to the outer limits of the
structure at the bottom, and shall slope outward at such angle as may be required
for stability of excavated face.

In rock, excavation shall be carried to a point six inches (6") outside the structure
so that no rock is left within six inches (6") of the finished structure.

Care shall be taken as the excavation approaches the desired grade to avoid
overdepth excavation and provide a firm and undisturbed soil surface on which
footings, slabs or foundations are to be placed. Should the CONTRACTOR
excavate below the desired grade level, the excavation shall be brought to grade
by the use of concrete or compacted crushed stone at the expense of the
CONTRACTOR. The use of tamped earth backfill under foundations, footings or
slabs will not be acceptable.

Where structures rest partially or wholly upon rock, the rock the rock shall be
excavated to a point six inches (6") below bottom of structure and compacted
crushed stone shall be used to bring the excavation back to grade, provided
however, that where the structure will rest completely on sound solid rock, the
ENGINEER may at his discretion permit the footing, foundation or slab to be placed
directly upon the rock surface. Where the CONTRACTOR is permitted to place
concrete directly on the rock, all dirt and weathered rock shall be removed and any
seams or crevices shall be cleaned and filled with grout or mortar prior to
placement of the structural concrete.

Should the material found at the desired subgrade appear to be unstable or
otherwise unsuitable for support of the structure such condition shall be
immediately called to the attention of the ENGINEER. The ENGINEER may direct
that such unsuitable material be removed and replaced with compacted crushed
stone, he may modify the foundation design to suit the condition, or he may
determine that the bearing capacity of the material is suitable for the load to be
supported; but in any case he shall provide written instructions to the
CONTRACTOR as to the procedure to be followed.

4.05 ROCK EXCAVATION

Rock excavation shall consist of loosening, removing and disposing of all rock
larger than nine (9) cu. ft. in volume, which in the opinion of the ENGINEERS can
only be removed by blasting or other equivalent methods. Such materials to be
classified as solid rock shall include boulders, bed rock, or solid concrete but shall
not include pavement or shaly materials that can be loosened by other methods.
Where rock excavation is encountered in trenches the excavation shall be carried to a depth of six inches (6") below the bottom of the pipe. The rock shall also be removed to a width of at least six inches (6") beyond the outside of the pipe on each side so that no rock is left within six inches (6") of the outside wall of the pipe.

Where rock is excavated in the bottom of the trench, the trench shall be brought back to grade by the use of crushed stone that shall be compacted to form a stable base for the pipe laying operation.

The CONTRACTOR shall exercise all necessary precaution in blasting operations. Suitable blasting mats shall be provided and utilized as required. Blasting shall be done only by experienced men. Careless shooting, resulting in the ejection of stones or other debris during blasting, shall be corrected immediately by the CONTRACTOR'S representative.

No blasting shall be done until the CONTRACTOR has taken out the necessary insurance to fully protect the OWNER from all possible damages resulting from the blasting operations. If blasting is required, an acceptable pre-blast survey shall be conducted.

The blasting shall be done in accordance with all recognized safety precautions and in accordance with regulations of authorities having jurisdiction.

Where rock is encountered in the immediate vicinity of gas mains, telephone cables, building footings, gasoline tanks, or other hazardous areas the CONTRACTOR shall remove the rock by means other than blasting. Care shall be taken in blasting operations to see that pipe or other structures previously installed are not damaged by blasting. In general, blasting shall not be done within twenty five feet (25') of the completed pipe line.

Excavated rock that cannot be utilized in trench backfill as permitted under Section 5.10 shall be removed from the site and disposed of as directed by the ENGINEERS.

4.06 SHEETING AND SHORING

The CONTRACTOR shall provide such bracing, sheeting or shoring as may be necessary for the protection of life and property, or the completed structure. Sheetin will be required where necessary to restrict the trench width to acceptable limits above the top of pipe.

Sheeting, shoring or bracing shall conform to applicable safety codes, and shall be left in place until the pipe is laid, checked and backfilled to a safe level at or above top of pipe. The bracing or sheeting may then be removed in an approved manner unless the ENGINEER specifically directs that the sheeting be left in place. Where the sheeting is left in place either at the direction of the ENGINEER or option of the CONTRACTOR, the sheeting shall be cut off at least eighteen inches (18") below the finished ground level.

Care shall be taken in removing sheeting to avoid weakening the trench, increasing the backfill load or endangering adjacent property. Voids left by the removal of sheeting shall be filled in and compacted with suitable material using tamps intended for this purpose.

4.07 REMOVAL OF WATER

The CONTRACTOR shall be responsible for handling run-off, ground water and
sewage in such a way as to maintain trenches and excavations in a dry condition until the work is completed. Pumps piping, well points, labor, fuel, and other facilities necessary to control, intercept, remove and/or dispose of water shall be provided by the CONTRACTOR at his own expense.

Water shall be kept out of trenches and other excavations to the extent necessary to protect the supporting strength of the foundation material, permit efficient and satisfactory assembly or replacement of facilities, and to prevent floating or misalignment.

Water removed from trenches or holes shall be discharged to natural drains in such a way as to avoid danger or damage to adjacent property owners or sewers.

Where the CONTRACTOR fails, refuses, or neglects to control water in trenches or other excavations, and corrective work is deemed by the ENGINEER to be necessary as a consequence thereof, such work shall be at the CONTRACTOR’S expense.

4.08 STORAGE OF EXCAVATED MATERIAL

Excavated material shall be deposited in such a manner as to avoid danger to workmen, sewer, or traffic, and to cause minimum inconvenience through blocking of drives, sidewalks, natural drains, etc. Where indicated on the drawings, or necessitated by conditions prevailing, the CONTRACTOR shall haul away and stockpile excavated material.

4.09 DISPOSAL OF SURPLUS EXCAVATED MATERIAL

Excavated material that is unsuitable or unnecessary for backfilling shall be removed from the job site and disposed of at the CONTRACTOR’S expense. The CONTRACTOR must not sell or give away surplus excavated material without first offering said material to the OWNER, but if the OWNER does not have a need for the material at a location within one mile of the job site, the CONTRACTOR shall make his own arrangements for disposal.

4.10 BACKFILL FOR TRENCHES

a. General

Backfilling of trenches will proceed as pipe laying progresses so that the trench will be filled in as rapidly as possible after the pipe has been assembled and inspected. The CONTRACTOR shall, however, afford the inspector ample opportunity for observing the assembled pipe line before placing the backfill and, if requested by the inspector shall delay the backfilling operation when the inspector is not present at the site. It is intended that the CONTRACTOR will backfill trenches and place base stone on the same day that the trench is excavated. All streets and walks shall be broomed to remove all earth and loose rock and shall be watered as necessary to prevent a dust problem.

Within 14 days of excavation, all excess material shall be removed and affected area shall be maintained in an acceptable condition.

Backfilling procedures will normally fall under two (2) categories as follows:

(1) Under highway, streets, drives and areas subject to traffic, either
under paving, or in unpaved areas (this category will include shoulders and driveways).

(2) Open fields or other areas not covered under Item 1.

b. **Backfill for Sewer Trenches**

In highways, streets, driveways, all areas subject to traffic, and certain areas as designated on the drawings, the backfill shall consist entirely of crushed stone which shall be placed in layers or lifts not exceeding twelve inches (12") in thickness and shall then be carefully compacted to maximum density or minimum volume. Number 57 crushed stone backfill shall be placed around the pipe six inches (6") below the bottom of the pipe and twelve inches (12") above the top of the pipe shall be placed by hand to avoid damage to or misalignment of the sewer. After the backfill has been placed to a depth of at least twelve inches (12") above the top of the pipe the additional crushed stone backfill may be placed by means of front end loaders, bulldozers or other suitable mechanical equipment subject to the twelve inch (12") limitation on maximum thickness of layers placed before compaction. Flowable fill mortar shall be placed in locations shown on the plans or as directed by Engineer. The flowable fill mortar shall be covered by necessary means i.e. steel plates or any other approved means while in the plastic state. Backfill shall not be placed on the flowable fill mortar prior to final set or hardening as determined by the engineer. Flowable fill mortar shall at no time come in direct contact with any utility lines. Flowable fill mortar shall commence 6-inch above top of pipe. Placement shall be in accordance with TDOT Standards for Road and Bridge Construction. For category two, the backfill up to a point twelve inches (12") above the top of the pipe shall be crushed stone and shall be placed by hand as specified in the preceding paragraph. The backfill for category two in areas not ordinarily subjected to traffic, may consist of suitable excavated material placed by machine after the backfill reaches a depth of twelve inches (12") over the top of the pipe, and the backfill shall be compacted by means of a suitable wheeled vehicle such as a tractor or front end loader running longitudinally along the trench. After the backfill has been compacted in this manner additional fill material placed by machine after the backfill reaches a depth of twelve inches (12") over the top of the pipe, and the backfill shall be compacted by means of a suitable wheeled vehicle such as a tractor or front end loader running longitudinally along the trench. After the backfill has been compacted in this manner additional fill material placed by machine after the backfill reaches a depth of twelve inches (12") over the top of the pipe, and the backfill shall be compacted by means of a suitable wheeled vehicle. No rock larger the 1/2" may be used in the top twelve inches (12") of the backfill. Top soil may be required on all lots or similar areas if suitable material is not available on site.

Backfill up to the spring line of the pipe shall be placed as pipe laying progresses in order to maintain proper grade and alignment. Additional backfill shall not be placed until after the pipe has been inspected by the ENGINEERS and approved for backfill.

In wide deep trenches the ENGINEER may at his discretion permit the use of rock larger than six inches (6") in the backfill, provided such rock is carefully placed in such manner that the final position of the rock will not be within the vertical prism lying directly over the pipe or within three feet (3') on either side of the pipe.

In all instances sufficient care must be exercised to avoid leaving any holes or voids over, around or under stones, boulders, or other backfill material that may later be filled by leaching or settlement of surrounding material.
thereby causing future trench settlement. Where the CONTRACTOR desires to use excavated rock for backfill material and such rock meets the dimensional requirements as specified herein, the CONTRACTOR shall provided additional backfill material of a suitable nature to fill the voids.

In locations not subject to traffic where excavated material is permitted in the backfill such material shall be brought up to the original ground level as indicated above and shall then be mounded over to provide for additional settlement. The CONTRACTOR shall exercise care to confine the mound to the area immediately over the trench and shall be responsible from time to time during the one year warranty period to fill in areas where excessive settlement has occurred.

The CONTRACTOR shall be responsible for and shall protect all sewers, storm sewers, and electric, telephone, water or other pipes or conduits against danger or damage while the trenches are being backfilled and from future settlement of the backfill. Where such damage should occur as a result of the CONTRACTOR’S operations, he shall repair such damage promptly to the ENGINEER’S satisfaction.

The CONTRACTOR’S attention is called to the fact that he will be held completely responsible for any damage to pavement, sidewalks, curbs, gutters, meter or valve boxes, street inlets, or other structure or appurtenances as a result of the CONTRACTOR’S operations. It should be specifically noted that the CONTRACTOR shall be responsible for damage even though the character or nature of the original pavement or structure was such that it was not capable of carrying the load of the construction equipment regardless of the construction methods used.
SECTION 5.1 - INSTALLATION OF SEWER PIPE AND RELATED ITEMS

5.1.01 GENERAL

The CONTRACTOR shall use only experienced men in the final assembly of pipe in the trench, and all pipe shall be laid in accordance with these specifications and the recommended practice of the pipe manufacturer. Trench bottoms shall be carefully prepared, shall be free of water and bedding as specified shall be in place.

Care shall be exercised to insure that pipe of the proper strength or classification meeting the specifications in every respect is provided at the site of pipe laying operations. Recommended tools, equipment, lubricant and other accessories needed for proper assembly or installation of the pipe shall be provided at the site of the work. Any damaged or defective pipe discovered during the pipe laying operations shall be discarded and removed from the site of the pipe laying operations.

Alignment and grade shall be carefully maintained during the laying operations. The method used for maintaining grade and alignment must be acceptable to the ENGINEERS and must produce the desired results. The top of the bedding material must be brought to the exact grade and must be shaped so as to provide effective support for the bottom quadrant of the pipe except at the bells.

5.1.02 HANDLING PIPE AND ACCESSORIES

The CONTRACTOR shall exercise care in the storage and handling of pipe, both on the storage yard and at the site of laying operations. Suitable clamps, slings, or other lifting devices shall be provided for handling pipe and fittings. Pipe and fittings shall be inspected for defects and for dirt or other foreign material immediately before placing them in the trench. Suitable swabs shall be available at the site of laying operations, and any dirt or foreign material shall be removed from the pipe before it is lowered into the trench.

5.1.03 LAYING DUCTILE IRON PIPE (DIP)

Where DIP is shown, specified or directed by the ENGINEER, the pipe shall be of the type and class as indicated. All DIP sewer pipe used on the project shall conform to provisions in Section 3.18 under MATERIALS. A minimum of six inches (6") of crushed stone bedding shall be placed in the bottom of the trench to provide continuous support of the bottom quadrant of the pipe. The CONTRACTOR shall bring the crushed stone bedding up to the required level to provide support of the bottom quadrant and shall then shape the bedding to receive the pipe. Bell holes shall be dug so that the bottom of the bells will not support the pipe.

After the bedding has been shaped and the pipe has been installed, crushed stone backfill shall be carefully placed by hand and compacted on both sides of the pipe and up to a level twelve inches (12") above the top of the pipe.

After the pipe has been cleaned and inspected for defects and lowered into the trench, the gasket shall be coated with lubricant of the type supplied by the pipe manufacturer and inserted in the groove provided for the purpose. The pipe shall then be assembled with due care being taken to insure that the spigot end of the pipe is shoved home and that the pipe is left in proper grade and alignment.
Whenever pipe laying operations are to be discontinued for a period of time exceeding two (2) hours, the end of the pipe shall be carefully secured to avoid displacement or misalignment and a tight fitting plug or stopper shall be place in the line. Upon resumption of laying operations, the plug or stopper shall not be removed from the line until any water, mud or other debris has been removed to avoid entry into the completed section of the sewer.

Unless otherwise indicated ductile iron pipe shall be laid with slip type compression joints, equal to the manufacturers standard for pressure water pipe and assembly of the joints shall be in accordance with manufacturer's recommendations using lubricant and accessories as provided by the pipe manufacturer.

Whenever it is necessary to cut a joint of pipe in order to fit the trench conditions, the cutting shall be done using the equipment as recommended by the manufacturer for the specific type of pipe involved. The cut shall be made so as to leave a smooth end at right angles to the axis of the bore and the end shall be beveled or finished as required to make the joint without risk of damage to the gasket.

In stream crossings, ravines, shallow cuts and other locations where the pipe will not be laid on bedding placed on original subgrade the pipe shall be supported on concrete piers as detailed on the drawings or as directed by the ENGINEERS. Piers shall be of Class A concrete with reinforcing as shown. The tops of piers shall be carefully set at the exact elevation and shall be shaped so as to provide support for the bottom half of the pipe with allowance being made for the outside diameter of the pipe plus the thickness of a layer of tarred felt around the outside of the pipe. After the concrete has obtained satisfactory strength the cast iron pipe may be installed across the piers using one or more layers of tarred felt between the surface of the concrete and the outside diameter of the pipe. The CONTRACTOR may, at his option, install the pipe to exact grade and alignment using temporary supports and then construct the permanent piers for the pipe, provided suitable precautions are taken to avoid any misalignment during the construction of the piers.

5.1.04 LAYING (PVC) SEWER PIPE

All PVC sewer pipe used on the project shall conform to provisions in Section 3.22 under MATERIALS. It is desired that trench widths from a point one foot above the top of the pipe down to the bottom of the trench be held to a minimum consistent with the provision of necessary space for proper assembly of the pipe. In general, it is not anticipated that the trench width will exceed the nominal pipe diameter plus eighteen inches (18").

A minimum of six inches (6") of crushed stone bedding shall be placed in the bottom of the trench to provide continuous support of the bottom quadrant of the pipe. The CONTRACTOR shall bring the crushed stone bedding up to the required level to provide support of the bottom quadrant and shall then shape the bedding to receive the pipe.

After the bedding has been shaped and the pipe has been installed, crushed stone backfill shall be carefully placed by hand and compacted on both sides of the pipe and up to a level twelve inches (12") above the top of the pipe.

After the pipe has been cleaned and inspected for defects and lowered into the trench, the mating surfaces of the compression joint shall be wiped clean and coated with lubricant of a type supplied by the pipe manufacturer. The pipe shall
then be assembled with due care being taken to insure that the spigot end of the pipe is shoved home and that the pipe is left in proper grade and alignment.

Whenever pipe laying operations are to be discontinued for a period of time exceeding two (2) hours, the end of the pipe shall be carefully secured to avoid displacement or misalignment and a tight fitting plug or stopper shall not be removed from laying operations, the plug or stopper shall not be removed from the line until any water, mud or other debris has been removed to avoid entry into the completed section of the sewer.

5.1.05 LAYING POLYETHYLENE SEWER PIPE

All polyethylene sewer pipe used on the project shall conform to provisions in Section 3.24 under MATERIALS. It is desired that trench widths from a point one foot (1’) above the top of the pipe down to the bottom of the trench be held to a minimum consistent with the provision of necessary space for proper assembly of the pipe. In general, it is not anticipated that the trench width will exceed the nominal pipe diameter plus eighteen inches (18”).

A minimum of six inches (6”) of crushed stone bedding shall be placed in the bottom of the trench to provide continuous support of the bottom quadrant of the pipe. The CONTRACTOR shall bring the crushed stone bedding up to the required level to provide support of the bottom quadrant and shall then shape the bedding to receive the pipe.

After the bedding has been shaped and the pipe has been installed, crushed stone backfill shall be carefully placed by hand and compacted on both sides of the pipe and up to a level twelve inches (12”) above the top of the pipe.

After the pipe has been cleaned and inspected for defects and lowered into the trench, the mating surfaces of the compression joint shall be wiped clean and coated with lubricant of a type supplied by the pipe manufacturer. The pipe shall then be assembled with due care being taken to insure that the spigot end of the pipe is shoved home and that the pipe is left in proper grade and alignment.

Whenever pipe laying operations are to be discontinued for a period of time exceeding two (2) hours, the end of the pipe shall be carefully secured to avoid displacement or misalignment and a tight fitting plug or stopper shall be placed in the line. Upon resumption of laying operations, the plug or stopper shall not be removed from the line until any water, mud or other debris has been removed to avoid entry into the completed section of the sewer.

All construction methods and procedures shall be in strict accordance with manufacturers recommendations. The CONTRACTOR shall furnish ten (10) "Cut-in" Service Tees for each size line furnished, in addition to the Tees set up in the Bid form.

5.1.06 COUPLINGS AND CONNECTIONS

Unless otherwise indicated or directed by the ENGINEERS, fittings shall be of the same material as the pipe line in which they are to be installed. Fittings shall be furnished with joints of the same type used throughout the rest of the pipe line unless such joint shall not be available and the ENGINEER should approve a substitute type joint. Fittings shall be of the type indicated on the drawings and shall be the manufacturers standard conforming to all applicable standard specifications and dimensional tolerances appropriate for the material of
construction. Fittings for PVC pressure pipe to be gray or ductile iron only. Couplings for gravity sewers are specified herein in Section 3.23.

Connections of pipes to manholes or other large structures shall be made using short lengths of pipe to avoid stressing the pipe at the point where it is placed in the wall of the structure.

Pipes entering or leaving masonry or concrete walls shall have one flexible joint located not more than 2'-0" in length with another flexible joint at the end of the 2'-0" pipe length in such a way as to provide for limited lateral or vertical movement of the pipe line as well as limited deflection. Ordinary compression type joints of the types specified for gravity sewers shall be considered as having sufficient flexibility for this purpose. The supplier of the pipe for the sewer lines shall furnish with the pipe order the required number of specials and short lengths of pipe for the CONTRACTOR to install the required flexible connections without improvising.

5.1.07 SPECIAL LAYING CONDITIONS

In wet or mucky areas where the subgrade or the trench walls have insufficient stability to support the installed sewer the CONTRACTOR will be directed to remove such unstable material and replace same with incompressible backfill. Where the wet or mucky condition is caused by the CONTRACTOR'S failure or neglect to properly handle ground water or protect against the entrance of storm water the CONTRACTOR will be required to remove and replace the unstable material at his own expense.

Cradle or encasement concrete shall be provided in locations as shown on the drawings or where the nature of the work requires such protection in the event the cradle or encasement concrete is required but is not shown on the drawings. The CONTRACTOR shall obtain written authorization from the ENGINEER for the installation of such protection which authorization shall also include the pay limits for the special protection.

5.1.08 MANHOLES

Consideration will be given to the use of either cast in place manholes or precast manholes on this project. In the event the CONTRACTOR elects to use precast manholes, he shall submit details of the proposed manholes together with the name of the supplier to the ENGINEERS for approval before any of the precast manholes are shipped to the job site.

a. PRECAST - Manholes may be used with precast floors, or with structural concrete floors poured in place. Precast risers shall be furnished with blocked out openings for pipes entering and leaving the manhole. Individual riser sections shall be furnished for the exact conditions to be encountered in the field and shall be constructed so as to suit field conditions and to line up properly with the pipes and manhole steps in other riser sections. Misalignment of steps or improperly located holes for incoming pipes shall be cause for rejection of the manhole sections. Precast manhole sections shall be joined together in such a way as to present a smooth uniform joint that shall be structurally sound and water tight.

b. CAST-IN-PLACE - Manholes shall be constructed in place in accordance with the details shown on the drawing in these specifications with forms equal to ABS plastic forms as marketed by Improved Construction Methods, Inc., P.O. Box 685, Jacksonville, Arkansas.
The base shall be cast monolithically with the rest of the manhole. The invert and flow channel shall be formed during or immediately after the placing of the concrete and brush-finished as soon as the concrete has sufficiently set.

The base concrete shall be 3,000 psi, maximum slump four inches (4”), vibrated or tamped on undisturbed bearing. The base shall have a minimum diameter eight inches (8”) greater than the outside diameter of the manhole, and a minimum thickness including the area under the pipe as follows:

<table>
<thead>
<tr>
<th>MANHOLE DEPTH</th>
<th>BASE DEPTH</th>
</tr>
</thead>
<tbody>
<tr>
<td>0’ to 8’ manhole</td>
<td>8”</td>
</tr>
<tr>
<td>8’ to 12’ manhole</td>
<td>10”</td>
</tr>
<tr>
<td>12’ &amp; deeper manhole</td>
<td>12”</td>
</tr>
</tbody>
</table>

All invert channels shall be smooth and accurately shaped to a semi-circular bottom conforming to the inside of the adjacent sewer section. Inverts shall extend up at least half of the diameter of the pipe. Changes in the direction of the sewer and entering branches shall have a true curve of as large a radius as the size of the manhole will permit.

The vertical forms, wall spacers, steps and placing cone must be carefully positioned and firmly clamped in place before any placement is made.

The wall spacers must be located ninety degrees (90°) from each other. The manhole shall be cast of 3,000 psi concrete with a maximum slump of four inches (4”). Concrete must be carefully vibrated on each side of each pipe as concrete is deposited in evenly distributed layers of about 18” with each layer vibrated to bond it to the preceding layer. The wall spacers must be raised as the placements are made with the area from which the spacer is withdrawn being carefully vibrated. Excessive vibration is to be avoided. A maximum of two percent (2%) Calcium Chloride may be added to the concrete, at the CONTRACTOR’S option, to speed the set. The forms may be removed as soon as the concrete has sufficiently set.

Form marks and offsets up to one inch (1") will be permitted on the outside surface of the manhole. Form marks and offsets up to one-half inch (1/2") will be permitted inside of the manhole. All offsets on the inside surface of the manhole will be smoothed and plastered so there is no projection or irregularity capable of scratching a worker or catching and holding water or solid materials.

Honeycomb will be plastered with a mortar consisting of three (3) parts of masonry sand to one (1) part Portland cement immediately upon removal of the forms.

The specific ring and cover and the method of installing it will be approved by the ENGINEER.

Manhole frames, covers and steps shall conform to Section 3.12 and 3.13 of these specifications. Manhole steps shall be staggered with even spacing of approximately sixteen inches (16”) between steps.

5.1.09 SERVICE CONNECTIONS

Sewer service lines shall be provided as shown on the Plans or as directed by the ENGINEERS. Service connections shall consist of tees or wyes with branch connection, curves and service pipe. Pipe and fitting joints shall be compression
type as used on the main sewer. Service pipe and fittings shall be of the same material as used for the main sewer. Service pipe shall be laid on a slope of at least 1/8" per foot. Sewer service lines shall conform to details as shown on the drawings and shall terminate at the property line with a tight compression stopper.

Vertical stacks as detailed on the drawings shall be used for service connections wherever the sewer depth exceeds eight feet (8') and only where directed by Engineer.

Services to be installed across State Highways, Railroads and other designated areas shall be installed by boring and jacking and six inch (6") PVC pipe shall be used where pipe is installed by boring and jacking.

In the event that it should be necessary to install a service connection where a tee has not been provided, saddles must be such a way as to effect a permanent water tight joint as recommended by the pipe manufacturer.

Excavation, laying and backfilling for service lines shall conform to the applicable specifications for main sewers.

The CONTRACTOR shall make connections to the existing sewers in accordance with details shown on the drawings and as described herein.

5.1.10 CONNECTIONS TO EXISTING SEWERS

New manholes shall be constructed over the existing sewer at points where the proposed sewer will connect and the top of the pipe shall be cut out to provide for flow channels for both existing and proposed sewers.

The CONTRACTOR shall make the necessary provisions to keep the existing sewer in operation without bypassing to the ditch or creek or ground surface.

5.1.11 CONCRETE

Concrete is to be proportioned in two classes according to use as follows:

Class "A" for reinforced concrete structures, non-reinforced portions of manholes control chambers and interceptor structures, curbs and gutter driveways, sidewalks and surface and base courses for highway and street paving.

Class "A" for encasement around sewers and branches and for cradle or refill under sewers and tunnel backfill.

Class "A" concrete is to be proportioned one 94 lb. sack Portland cement, 195 lbs. sand, and 270 lbs. coarse aggregate. These proportions may be varied by the ENGINEERS after the materials supplied have been tested and proportions for the greatest density and workability determined provided that no more than 7.25, nor less than 6.0 bags of cement per cubic yard of concrete will be required.

Class "A" concrete shall have a minimum compressive strength of 4,000 lbs. per square inch in 28 days.

Class "B" concrete shall have a minimum compressive strength of 2,000 lb. per square inch and shall contain not less than 4.5 sacks of cement per cubic yard of concrete. The relative amounts of fine and coarse aggregate shall be comparable to that for Class "A" concrete.
The water used in mixing must be minimum required for a plastic mix. No water will be permitted to be used for purpose of hastening mixing and reducing of tamping and vibration.

The water content that is allowed will be at all times subject to regulations by the ENGINEERS.

In the case of Class "A" concrete, not more than 5-1/2 gallons of water to the bag of cement will be allowed in mixing concrete (or proportionately less when slump is above 4" and/or mix is wet) except in cases where, in the judgment of the ENGINEERS, additional water is necessary to obtain proper results.

Batching equipment shall include scales for weighing contents of wheelbarrows and a device for accurately measuring water by the gallon, to be used for proportioning each batch.

In case of ready-mixed concrete, specifications for proportioning of mixes shall be the same, except from manufacturer's experience with his own aggregates whereas he shall vary proportions of sand and coarse aggregates for the greatest density and workability of mix.

Prior to actual delivery of concrete, and at any change of proportioning, the manufacturer shall furnish a statement to the ENGINEERS giving the proportions of weight (dry) or cement, and of fine and coarse aggregates, that will be used in the manufacture of each mix ordered. Proportions must be approved by the ENGINEERS. Otherwise, proportioning of mix and batching plant shall be according to ASTM Designation C94 (latest revision) specifications for Ready-Mixed Concrete.

Forms of concrete with exposed surfaces shall consist of dressed and sized lumber, or metal, and must match on edges sufficiently to prevent leakage of mortar. Forms shall be built to such accuracy and braced to such and extent that they shall not vary from true lines and surfaces, where exposed, more than 1/4" before pouring concrete, nor more than 3/4" after pouring. Angle strips (3/4" size) shall be placed in all exposed corners of forms.

All steel reinforcement shall be delivered in new condition, either clean or with only a slight coating of rust. If stored on the works it must be kept under shelter or supported at least twelve inches (12") above the ground to prevent its becoming coated with dirt and when placed in forms it must be free from scale or dirt.

When placing in forms, it must be tied together to form a rigid frame before pouring concrete and must be secured in the walls of slabs in such a manner as to insure its holding the position designed for it in the finished work, by use of form stands, steel or concrete chairs or spacers. As a rule, steel bars must have a covering of 1-3/4" of concrete unless otherwise noted on the Plans. All splices shall be 36 bar diameters long and 1" between spliced bars.

Concrete shall be thoroughly mixed at least two minutes after all materials, including water are in the mixer drum having a capacity of at least one (1) sack batch.

Concrete must be poured into forms slowly enough to permit all thorough tamping and vibrating to eliminate any honeycombed surfaces.
Concrete pouring will not be permitted under conditions where there is danger of freezing, or when materials are frozen. After pouring, concrete must be protected from freezing weather for at least 72 hours.

Ready-mixed concrete delivery facilities pledged to the concrete pour shall be approved by the ENGINEERS before permission will be given to start the pour.

The period between termination of placing by one truck and starting by the next shall not be longer than ten (10) minutes at temperatures above 70°F, nor longer than 20 minutes below 70°F. The concrete in a truck mixer or agitator must be totally discharged with 1-1/2 hours after the introduction of mixing water to the cement and aggregates. The mixing operation shall begin within thirty (30) minutes after the cement has been intermingled with the aggregates.

Otherwise, mixing, mixers, agitators and inspection shall be according to ASTM Designation C94 (latest revision) Specifications for Ready-Mixed Concrete. Non-agitating trucks for hauling concrete from central mixing plant will not be accepted.

After the removal of the forms, all surfaces that will show in the finished work shall be immediately rubbed down with a coarse carborundum stone or wooden float (if concrete is soft enough for the use of the wood) and left in this condition until concrete has thoroughly hardened. At such time as there is no longer any danger of its subsequent damage from the progress of the work these exposed surfaces must be rubbed with a fine carborundum stone until the finish is similar and equal to that required by the State Highway department for bridges and railing surfaces. Cement or mortar coating will not be permitted. Rubbing is not required below ground.

All concrete must be kept wet or moist for a period of at least forty eight (48) hours after pouring, in order to prevent too rapid drying out.

In dry weather wooden forms must be thoroughly wet before concrete is placed in them and must also be kept in this condition during the period above mentioned. Concrete must be covered and kept damp to protect it from the sun as soon as the surfaces are firm enough to allow the placing of such covering or protection.

At least one slump test shall be made before first concrete pour, at start of pouring any concrete and at each five (5) cubic yards deposited during one operation. These shall be made from same samples as those taken for cylinder tests and records of same kept therewith. Tests shall be made according to ASTM-C143 and as required under ASTM Designation C94 for Ready-Mix Concrete. Mix is designed for a slump test of two inches (2") and not more than four inches (4"), except in cases where thin sections would indicate, in the opinion of the ENGINEERS, that a wetter mix is more desirable. The CONTRACTOR shall furnish necessary equipment for the slump tests.

Ordinarily on sewer and water line jobs requiring only small amounts of concrete per pour, the cylinder tests will be waived. However, should the ENGINEERS have reason to doubt that the concrete being furnished meets the strength specifications, they shall have the right to order cylinder tests according to the following specifications.

At the start of concreting, or before if practical, the CONTRACTOR shall make from a single batch a set of four (4) cylinders per ASTM C31. Two shall be tested seven (7) days and two at twenty eight (28) days per ASTM C-39.
At each time when 20 or more cubic yards of concrete are placed during one operation, and when the sum of smaller deposits of concrete equal 30 cubic yards since previous test, and at any change in mix, four (4) cylinder tests will be required, two tested at seven (7) days, and the other two at twenty nine (29) days per ASTM C39. In case of Ready-Mix Concrete, requirement for testing of ASTM Designation C94 and C172 shall be added.

The CONTRACTOR shall furnish all equipment for sampling and curing on the job and shall bear cost of laboratory curing and testing.
SECTION 5.2 – INSTALLATION OF REPURIFIED PIPE AND RELATED ITEMS

5.2.01 GENERAL

1.1 Contractor shall be responsible for safely storing materials needed for the work that have been accepted by him until they have been incorporated into the completed project. Keep the interiors of all pipes, fittings, and other accessories free from dirt and foreign matter at all times.

1.2 Wherever reaction blocking is necessary, it shall be considered an integral part of the water line work, and no separate payment shall be made for it.

1.3 Pipe sizes 4” – 10” diameter shall be PVC in conformance with AWWA C900 standards and specifications, unless specified otherwise by the Town.

1.4 Pipe sizes 12” and greater shall be ductile iron meeting the design thickness as specified by ANSI A21.50/AWWA C150 and meet the minimum pressure class of 300 psi, unless specified otherwise by the Town. The ductile iron pipe shall be centrifugally cast in metal or sand-lined molds and conforming to ANSI A21.51/AWWA C151 specifications. Pipe joints shall meet ANSI A21.11/AWWA C111. Ductile iron pipe shall contain cement lining in accordance with ANSI A21.4/AWWA C104.

5.2.02 DUCTILE IRON PIPE AND FITTINGS

A. Ductile cast iron pipe shall be made of good quality ductile iron that meets the requirements for nodular iron castings of ASTM E8. It shall be plain end ductile iron pipe with push-on, single gasket joints. The design thickness shall be that specified by ANSI A21.50/AWWA C150 except that all pipe with a diameter of 12 inches or less shall have a wall thickness of 0.25 inches and all pipe with a diameter of 14 inches or more shall have a thickness of 0.28 inches or greater.

B. Ductile iron pipe shall be centrifugally cast in metal or sand-lined molds and shall conform to the specifications of ANSI A21.51/AWWA C151. It shall be made and tested in accordance with ASTM A339 and shall be subjected to and able to withstand a hydrostatic pressure of 500 psi. The maximum depth of pits shall be half that allowed in the AWWA specifications.

C. The push-on, single gasket joints shall be either Fastite (manufactured by American Cast Iron Pipe Company), Tyton (U. S. Pipe and Foundry Company), Super Bell-Tite (Clow Corporation), or other joints of similar type and equal quality. They shall be UL approved and able to withstand 200 psi of operating pressure.

D. The bell of each pipe shall have a tapered annular opening and a cast or machined retaining groove for the gasket. The gasket groove shall have a flared design so that maximum deflection will be provided. The plain spigot end of the pipe shall be beveled in order to simplify its entry into and centering within the bell and the compression of the gasket.

E. The gasket shall be of high quality vulcanized rubber made in the form of a solid ring to exact dimensions. The design of the gasket groove in the bell of the pipe and the design, hardness, and other properties of the gasket itself
shall be such that the joint is liquid tight for all pressures from a vacuum to a maximum rating of 350 psi of internal liquid pressure.

F. Enough lubricant shall be furnished with each order to provide a thin coat on the spigot end of each pipe. This lubricant shall be approved for being in contact with potable water and shall be nontoxic, impart no taste of smell to the water, and have no harmful effect on the rubber gasket. It shall have a consistency that will allow it to be easily applied to the pipe in either hot or cold weather and that will enable it to adhere to either wet or dry pipe. In no case shall lubricant other than that supplied by the pipe manufacturer be used.

G. Standard and special fittings shall be ductile iron. Use standard mechanical joint fittings. All fittings shall conform to the specifications of ANSI A21.10/AWWA C110.

H. Pipe and fittings shall be lined with enameline or a thin cement lining as specified in ANSI A21.4/AWWA C104. In addition, a bituminous seal coat or asphalt emulsion spray coat approximately 1 mil thick shall be applied to the cement lining in accordance with the pipe manufacturer’s standard practices.

I. Fitting laying lengths shall conform to ANSI A21.10/AWWA C110, short body.

J. Fittings shall be in accordance with the standard mechanical joint fittings manufactured by the U.S. Pipe and Foundry Company, American Cast Iron Pipe Company, Clow Corporation, or equal.

K. The pipe manufacturer is to furnish the A/E a certificate of inspection, sworn to by the factory inspector in the presence of a notary public, stating that the pieces of pipe in the shipment were made and tested in accordance with ANSI A21.51 and that they were subjected to and withstood a hydrostatic pressure of 500 psi. Each statement is to give the number of pieces of pipe in the shipment, the length of each piece of pipe, and the serial number of each piece of pipe making up the shipment. In addition, the weight of each individual piece of pipe making up the shipment is to be listed opposite the serial number of each pipe length and attached to the certificate of inspection.

L. If installing reclaimed/repurified water lines, the ductile iron pipe shall have the message “Caution - Repurified Water – Do Not Drink” stamped on opposite sides of the pipe at 180 degrees apart.

N. If installing reclaimed/repurified water lines, a continuous 3-inch wide non-detectable warning adhesive tape reading “Caution – Repurified Water Line Below” shall be wrapped around the line at 12” spacing.

5.2.03 GATE VALVES - Valves on water lines ten inches and smaller shall be of double disc, parallel seat, iron body bronze mounted type or resilient wedge, iron body, iron gate with bond-in-place Nitrile elastomer designed to work equally well with pressure on either side of the gate. All gate valves shall be in accordance with or exceed AWWA C500. Working pressure shall be 200 psi.

Valves ten inches and smaller shall be Mueller A2380-20, American Darling No. 55, Clow F-5065, or American Flow Control Series 2500, or equal, with mechanical joints. All gate valves shall be resilient seated, manufactured to meet or exceed the requirement of AWWA C509 latest revision. Valves shall be suitable for
installation in an approximate vertical position in buried pipelines. Stem seal shall consist of three (3) 0-ring seals. All valves shall open to the left (counterclockwise) with non-rising stems and shall be provided with a 2-inch square operating nut. All internal and external exposed surfaces shall be fusion-bonded epoxy coated with an approved epoxy coating to a minimum thickness of 6 mils, complying fully with AWWA 550 and certified to NSF61. Bodies shall be constructed of cast iron (ASTM A126, Class B) and shall have integrally cast mechanical joint ends in accordance with AWWA C111. Accessories (bolts, glands, and gaskets) shall be supplied by the valve manufacturer. Valves shall be steel body of molded-in vulcanized Buna-N bonded to the valve body. Valves shall be furnished with mechanical joint ends in accordance with ANSI A21.11 unless otherwise shown or directed.

Valves shall be complete when shipped and the manufacturer shall use due and customary care in preparing them for shipment so as to avoid damage in handling or in transit. Particular care shall be taken to see that all valves are completely closed before shipment.

5.2.04 BUTTERFLY VALVES - Valves on water lines 12 inches and larger shall be butterfly valves, be designed for direct burial service, and meet or exceed performance requirements for water application of applicable standards such as AWWA C504. Valves shall be fitted with operators designed to accept Metro Valve Box "John Bouchard & Sons Company, No. 8006"; valves shall open to the left. All butterfly valves shall be of the tight closing, rubber-seat type. Valves shall be bubble-tight at rated pressures in either direction, and shall be satisfactory for applications involving throttling service and/or operation and for applications involving valve operation after long periods of inactivity. Valve discs shall rotate 90° from the full open position to the tight shut position. Regardless of valve size, angular mis-position of disc can be 1° off center without leakage. The manufacturer shall have manufactured tight-closing, rubber-seat butterfly valves for a period of at least five (5) years.

All valve bodies shall be cast iron ASTM A-126 Class B, narrow body design. Flange drilling shall be in accordance with ANSI B16.1 standard for cast iron flanges. Body thickness shall be in strict accordance with AWWA C504 where applicable.

All valve discs shall be constructed either of cast iron ASTM A-48 with stainless steel seating edge or ductile iron ASTM A-536 with stainless steel seating edge. The disc shall not have any hollow chambers that can entrap water. All surfaces shall be visually inspected and measurable to assure all structural members are at full design strength. Disc and shaft connection shall be made with stainless steel pins.

Valves shall be Dresser Manufacturing Company No. 450 butterfly valve, Henry Pratt Company "Groundhog," American Darling Class 150B, or equal.

All shafts shall be turned, ground and polished and constructed of 18-8 Type 304 or Type 316 stainless steel. Shafts shall be two-piece, stub-type keyed for operator connection. Shaft diameters shall meet minimum requirements established by latest revision of AWWA Standard C504 for their class where applicable.

All seats shall be of a synthetic rubber compound. Seats shall be a full 360° without interruption and have a plurality of grooves mating with a spherical disc edge-seating surface. Valve seats shall be field adjustable around the full 360° circumference and replaceable without dismantling operator, disc or shaft and
without removing the valve from the line. Manufacturer shall certify that rubber seat is field replaceable.

All valves shall be fitted with sleeve-type bearings. Bearings shall be corrosion resistant and self-lubricating. Bearing load shall not exceed 1/5 of the compressible strength of the bearing or shaft material.

Valve operators shall conform to AWWA Standard C504 and shall be designed to hold the valve in any intermediate position between full open and fully closed without creeping and fluttering.

Hydrostatic and leakage tests shall be conducted in strict accordance with AWWA Standard C504.

5.2.05 TAPPING SLEEVES AND VALVES - Tapping sleeves shall consist of a cast iron body with removable bolts, mechanical joint gasket design giving 360° pipe coverage and cast iron flange to accept standard tapping valve. Tapping sleeve shall be Muller H-615 or approved equal. Tapping valve shall conform to all applicable specifications for gate valves and shall be manufactured as tapping valves with line up groves. Tapping valve shall be Mueller T-2360, T-2361 or approved equal.

5.2.06 VALVE BOXES – Valve boxes for the water distribution system shall be made of concrete as shown in the standard details and shall be of the heavy roadway type. Base section shall be enlarged to enclose and protect valve-operating nut without being in contact with the valve or the pipe at any point. Top section shall be adjustable for elevation. Backfill around valves and box shall be tamped to maintain proper alignment of the box. Valve boxes that are not plumb or not properly centered will not be accepted.

All valve boxes shall be provided with covers on which the word “WATER” is cast in raised letters. Boxes shall be suitable for installation on mains laid at depths specified. Additional compensation will not be provided for deeper valve boxes made necessary by installation of mains at depths greater than minimum depths specified.

Valve boxes shall have an inside opening of not less than 11” by 13”. Standard precast reinforced concrete boxes having the same opening shall be provided. Concrete boxes and footing blocks shall be made of 4,500 psi concrete by an approved manufacturer. Cast iron valve boxes shall be as manufactured by an approved supplier shall require prior approval before installation.

5.2.07 FIRE HYDRANTS - Fire hydrants shall be iron bodied fully bronze mounted, hydrants manufactured to equal or exceed AWWA Standard C502-54. Hydrants shall be suitable for 150 psi working pressure and shall be subjected to a test pressure of 300 psi. Inlet connection shall be 6” mechanical joint. Main hydrant valve shall be compression type, closing with the pressure, with 5 1/4” valve opening. The hydrant shall have a 7.5” I.D. barrel. The bronze seat shall be threaded into mating threads of bronze for easy field removal.

All hydrants shall be equipped with two (2) 2 1/2” hose nozzles, one 4 1/2” pumper nozzle, breakable safety flange, and stem coupling. Threads on hose and steamer nipples, operating nut, and cap nuts shall conform to local Water Department standards. Bronze nozzles shall be securely locked to prevent them from blowing off. Hose and pumper nozzles shall be field replaceable. Hose threads shall be National Standard. Nozzle caps shall be equipped with non-kink chains.
Hydrants shall be of the "dry head" type with an oil or grease reservoir and provision for automatic lubrication of stem threads and bearing surfaces each time the hydrant is operated. Double O-ring seals shall be provided to keep water out of the hydrant top. Operating nut style shall be 1½” pentagon with direction of opening to the left and shall be equipped with a weather cap. The operating nut, main stem, coupling and main valve assembly shall be capable of withstanding input torque of 200 ft/lbs., in opening or closing directions. The hydrant shall open by being turned to the left and be so marked on the bonnet in cast letters with an arrow.

Hydrants shall be provided with multi-port drain ports. Drain valves operated by springs or gravity will not be acceptable. A positive stop shall be provided on the operating stem to prevent over travel when operating valve. It should not be necessary to excavate to repair or inspect internal parts. It should be removable without disturbing line joint or nozzle section of hydrant. Fire hydrants shall be supplied with a bituminous coating for buried portion of hydrant and a purple enamel finish for above ground portions of the hydrant.

Hydrants shall be Mueller "Centurion" or approved equal.

5.2.08 CASING PIPE - The casing pipe shall be of steel meeting the latest approved American Railway Engineering Association Specifications for “Pipelines for Carrying Flammable and Nonflammable Substances”. Casings shall be pipe conforming to the requirements of ASTM Designation A-139 Ductile Iron Pipe Class 250. The steel casing pipe shall have a minimum yield strength of 35,000 PSI and shall have the minimum wall thickness shown in the following table:

### TABLE OF MINIMUM WALL THICKNESS FOR STEEL CASING PIPE FOR E72 LOADING

<table>
<thead>
<tr>
<th>Carrier Pipe Diameter</th>
<th>Casing Pipe Diameter</th>
<th>Nominal Thickness</th>
</tr>
</thead>
<tbody>
<tr>
<td>4&quot;</td>
<td>12&quot;</td>
<td>0.250”</td>
</tr>
<tr>
<td>6&quot;</td>
<td>18&quot;</td>
<td>0.312”</td>
</tr>
<tr>
<td>8&quot;</td>
<td>20&quot;</td>
<td>0.375”</td>
</tr>
<tr>
<td>10&quot;</td>
<td>24&quot;</td>
<td>0.375”</td>
</tr>
<tr>
<td>12&quot;</td>
<td>24&quot;</td>
<td>0.375”</td>
</tr>
<tr>
<td>14&quot;</td>
<td>26&quot;</td>
<td>0.500”</td>
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<tr>
<td>16&quot;</td>
<td>30&quot;</td>
<td>0.500”</td>
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<tr>
<td>18&quot;</td>
<td>30&quot;</td>
<td>0.500”</td>
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<td>30&quot;</td>
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<tr>
<td>36&quot;</td>
<td>54&quot;</td>
<td>0.625”</td>
</tr>
<tr>
<td>42&quot;</td>
<td>60&quot;</td>
<td>0.625”</td>
</tr>
<tr>
<td>48&quot;</td>
<td>66&quot;</td>
<td>0.625”</td>
</tr>
</tbody>
</table>

When the casing pipe is installed without benefit of a protective coating, the wall thickness shown above shall be increased to the nearest standard size, which is a minimum of 1/16 inches greater than the thickness shown.
5.2.09 PVC PIPE

A. All plastic pipe shall be made from Class 12454-B polyvinyl chloride plastic (PVC 1120) as defined by ASTM D1784 as defined by ASTM D1784. All PVC pipe shall meet the specifications of AWWA C900 (DR-14) with a minimum pressure class of 200 psi.

B. All Class 200, 250, or 315 pipe shall have NSF approval and be manufactured in accordance with ASTM D2241. The following tests shall be run for each machine on each size and type of pipe being produced, as specified below:

1. Flattening Test: once per shift in accordance with ASTM D2412. Upon completion of the test, the specimen shall not be split, cracked, or broken.

2. Acetone Test (Extrusion Quality Test): once per shift in accordance with ASTM D2152. There shall be no flaking, peeling, cracking, or visible deterioration on the inside or outside surface after completion of the test.

3. Quick Burst Test: once per 24 hours in accordance with ASTM D1599.

<table>
<thead>
<tr>
<th>SDR</th>
<th>Pressure Rating</th>
</tr>
</thead>
<tbody>
<tr>
<td>14</td>
<td>200</td>
</tr>
<tr>
<td></td>
<td>985</td>
</tr>
</tbody>
</table>

4. Impact Tests: for 6 inches and larger, once per shift in accordance with ASTM D2444; for 4 inches and smaller, once each two hours in accordance with ASTM D2444.

5. Wall Thickness and Outside Dimensions Tests: once per hour in accordance with ASTM D2122.

6. Bell Dimension Test: once per hour in accordance with ASTM D3139.

C. If any specimen fails to meet any of the above mentioned tests, all pipe of that size and type manufactured between the test periods must be scrapped and a full set of tests rerun.

D. Furnish a certificate from the pipe manufacturer stating that he is fully competent to manufacture PVC pipe of uniform texture and strength and in full compliance with these specifications and further stating that he has manufactured such pipe and done so in sufficient quantities to be certain that it will meet all normal field conditions. In addition, the manufacturer's equipment and quality control facilities must be adequate to ensure that each extrusion of pipe is uniform in texture, dimensions, and strength. Also furnish a certificate from the manufacturer certifying that the pipe furnished for this project meets the requirements of these specifications.

E. All pipe shall be manufactured in the United States of America. All pipe for any one project shall be made by the same manufacturer.

F. All 4 inches and 6 inches pipe may be furnished in the manufacturer's standard laying lengths of 20 feet, 38 feet, or 40 feet. Pipe 8 inches and larger shall be furnished in 20 feet lengths. The Contractor's methods of storing and handling the pipe shall be approved by the A/E. All pipe shall be
supported within 5 feet of each end; in between the end supports, there shall be additional supports at least every 15 feet. The pipe shall be stored away from heat or direct sunlight. The practice of stringing pipes out along the proposed water line routes will not be allowed.

G. Certain information shall be applied to each piece of pipe. At the least, this shall consist of:

1. Nominal size
2. Type of material
3. SDR or class
4. Manufacturer
5. NSF Seal of Approval

H. Pipe that fails to comply with the requirements set forth in these specifications shall be rejected.

I. The pipe shall have push-on joints designed with grooves in which continuous molded rubber ring gaskets can be placed. Gaskets shall be made of vulcanized natural or synthetic rubber; no reclaimed rubber will be allowed. Gasket materials shall meet the requirements of ASTM F477. The gaskets shall be of the manufacturer’s standard design dimensions and of such size and shape as to provide a positive seal under all combinations of joint and gasket tolerance. The gasket and annular groove shall be designed and shaped so that when the joint is assembled, the gasket will be radially compressed to the pipe and locked in place against displacement, thus forming a positive seal.

J. The spigot end of each pipe shall be beveled so that it can be easily inserted into the gasket joint, which in turn shall be designed so that the spigot end may move in the socket as the pipe expands or contracts. The spigot end shall be striped to indicate the distance into which it is to be inserted into the socket. Each joint shall be able to accommodate the thermal expansions and contractions experienced with a temperature shift of at least 75 degrees F.

K. Enough lubricant shall be furnished with each order to provide a coat on the spigot end of each pipe. This lubricant shall be approved for being in contact with potable water and shall be nontoxic, impart no taste or smell to the water, have no harmful effect on the gasket or pipe material, and support or promote any bacterial growth. The lubricant containers shall be labeled with the manufacturer’s name. In no case shall lubricant other than that supplied by the pipe manufacturer be used.

L. Joints shall be manufactured in accordance with ASTM D3139 except that the thickness of the bell shall be, as a minimum, equal to that of the barrel. Joints shall be either integral bell and ring joints with rubber compression gaskets as manufactured by the Clow Corporation, Johns-Manville, or Vulcan Plastic Corporation; twin gasket couplings as manufactured by the Certain-Teed Products Corporation; or equal. However, the pipe and bell must be made by the same manufacturer.

M. Standard and special fittings shall be gray iron or ductile iron. Use standard mechanical joint fittings. All fittings shall conform to the specifications of ANSI A21.10/AWWA C110. The gaskets shall be ducked tipped transition gaskets for use with PVC pipe.
N. Fittings shall be lines with enameline or a thin cement lining as specified in ANSI A21.4/AWWA C104. In addition, a bituminous seal coat or asphalt emulsion spray coat approximately 1 mil thick shall be applied to the cement lining in accordance with the pipe manufacturer’s standard practices.

O. Fitting laying lengths shall conform to ANSI A21.10/AWWA C110.

P. Fittings shall be in accordance with the standard mechanical joint fittings manufactured by the U.S. Pipe and Foundry Company, American Cast Iron Pipe Company, Clow Corporation, or equal.

Q. If installing reclaimed/repurified water lines, the PVC pipe shall be colored purple and have the message “Caution - Repurified Water – Do Not Drink” stamped on opposite sides of the pipe at 180 degrees apart.

R. If installing reclaimed/repurified water lines, a continuous 3-inch wide nondetectable warning tape reading “Caution – Repurified Water Line Below” shall be installed over the repurified water lines no less than 12 inches above the line.

5.2.10 METER AND SERVICE INSTALLATIONS

A. The service assembly shall include a corporation cock, copper service pipe gooseneck, meter yoke, meter, meter box, and tapping saddle as required.

B. The corporation cock shall be of solid bronze suitable for a compression flange on the service pipe and for tapping into the water main at a vertical angle. This cock shall be similar to Mueller H-15000, or equal. The threads on the corporation cock shall be Mueller.

C. Service pipe shall be 3/4 inch Type K copper meeting ASTM B88. Goosenecks shall be a minimum of 3 feet long. All cooper shall be wrapped at 12” spacing with adhesive purple locator tape that has the message “Caution - Repurified Water – Do Not Drink”

D. Meter yokes 5/8 inch x 3/4 inch with compression fittings shall be Ford Retro 2CVBHHHR with a Ford C38-23-2-5 3/4 inch tailpiece. Meter yokes for 1 inch meters shall be Badger VBHH44-10WR and the tailpiece shall be Ford C38-44-2-625. Meter yokes for 1-1/2” shall be Ford VBHH46-14R and 2” shall be Ford VBhh47-14R. Note, 1 ½” and 2” meter yokes must not have a bypass line. Each assembly shall include a meter check valve. Each meter yoke shall have an integral angle stop and provisions for locking

E. All meters shall be frost proof, sealed register, displacement type with bronze cast and made by Badger Meters, Inc. Meters shall be straight reading in gallons. Meters 1 inch in size shall be Badger M70. Meters 1-1/2 inch in size shall be Badger M120 B81. Meters 2 inches in size shall be Badger M170 B81. Larger meters shall have flanged connections and shall be Badger M25. All meter housings and indexes shall be colored purple.

F. Meter boxes for 5/8 inch x 3/4 inch assemblies shall be colored purple plastic meter boxes, Carson MSBCF 1324 18XL and MSBCF 1760 18XL. Larger meter boxes may be required for different applications and larger meters and will require approval from the Town of Smyrna Water Department prior to ordering or installing the meter boxes.
5.2.11 INSTALLATION OF WATER LINES

A. Lay water lines to and maintain at the lines and grades required by the drawings. All fittings, valves, and hydrants shall be at the required locations, the spigots centered in the bells, and all valves and hydrant stems plumb.

B. Unless otherwise indicated by the drawings, all water pipes shall have at least 30 inches of cover. No departure from this policy shall be made except with the approval of the A/E.

C. Provide and use tools and facilities that are satisfactory to the A/E and that will allow the work to be done in a safe and convenient manner. All pipe, fittings, valves, and hydrants are to be unloaded from the trucks using suitable tools and equipment. Use a derrick, ropes, or other suitable tools or equipment to lower all pipe, fittings, valves, and hydrants into the trench one piece at a time. Lower each piece carefully so that neither it nor any protective coating or lining it may have will be damaged. Under no circumstances drop or dump water line materials into the trench.

D. Any pipes strung out along the route of the proposed lines before the actual installation of those lines is due to take place shall not be lowered into the trench until they have been swabbed to remove any mud, debris, etc., that may have accumulated within them. PVC pipe shall be strung out a maximum of one day ahead of pipe laying. Remove all unnecessary material from the bell and spigot end of each pipe. Before any pipe is laid, brush and wipe clean the outside of its spigot end and the inside of its bell, and leave dry and oil-free.

E. Take every precaution to keep foreign material from getting into the pipe while it is being placed in the line. If the crew laying the pipe cannot put it into the trench and in place without allowing earth to get inside, then put a heavy, tightly woven canvas bag of suitable size over each end of the pipe, and leave in place until it is time to connect that pipe to the one adjacent to it.

F. Place no debris, tools, clothing, or other materials in the pipe during laying operations.

G. After a length of pipe has been placed in the trench, center the spigot end in the bell of the adjacent pipe, and then insert to the depth specified by the manufacturer and bring to the correct line and grade. Secure the pipe in place by tamping an approved backfill material around it.

H. Bell holes shall be big enough so that there is ample room for the pipe joints to be properly made. Between bell holes, carefully grade the bottom of the trench so that each pipe barrel will rest on a solid foundation for its entire length.

I. Whenever pipe laying is not in progress, close the open ends of pipe either with a watertight plug or by other means approved by the A/E. If the joints of any pipe in the trench cannot be completed until a later time, caulk them with packing in order to make them as watertight as possible; this shall be done not only at the end of each working day but also before work is stopped for lunch periods, bad weather, or any other reason. If there is water in a trench, leave this seal in place until the trench has been pumped completely dry.
J. Cut pipe so that valves, fittings, or closure pieces can be inserted in a neat and workmanlike manner and without any damage to the pipe. Follow the manufacturer's recommendations concerning how to cut and machine the ends of the pipe in order to leave a smooth end at right angles to the pipe's axis.

K. Lay pipe with the bell ends facing in the direction of laying unless otherwise directed by the A/E.

L. Wherever pipe must be deflected from a straight line (in either the vertical or horizontal plane) in order to avoid obstructions or plumb stems, or wherever long radius curves are permitted, the amount of deflection shall not exceed that necessary for the joint to be satisfactorily made, nor that recommended by the pipe manufacturer, and shall be approved by the A/E.

M. Lay no pipe in water or when it is the A/E's opinion that trench conditions are unsuitable. If crushed stone is used to improve trench conditions or as backfill for bedding the pipe, its use is considered incidental to the project, and no separate payment will be made for its use.

N. Where a water line crosses over a sanitary sewer, use a full joint of pipe with a standard mechanical joint, and center over the sewer and seal the open ends. Where a water line is to be parallel to a sanitary or storm sewer, lay it at least 10 feet from the sewer. If it is not practical for the water and sewer lines to be separated as described above, then lay the water line at least 18 inches above the top of the sewer.

O. Joint all pipe in the exact manner specified by the manufacturer of the pipe and jointing materials.

P. For detection purposes, a 12 gage solid strand copper tracing wire (shielded) or an approved metallic tape shall be installed as per the manufacturer's instructions. Connections between wires shall be soldered or connected with wire nut fasteners and wrapped.

5.2.12 HYDROSTATIC TESTS

A. Pressure Test

1. After pipe has been laid and backfilled as specified above, subject all newly laid pipe or any valved section thereof to a pressure of 200 psi. All services are to be laid prior to testing the main and tested as part of the test of the main.

2. The duration of each pressure test shall be at least two (2) hours.

3. Slowly fill each valved section of pipe with water, and apply the specified test pressure (based on the elevation of the lowest point of the line or section under test and corrected to the elevation of the test gauge) with a pump connected to the pipe in a manner satisfactory to the A/E. Furnish the pump, pipe, connections, gauges, and all necessary apparatus.

4. Before applying the specified test pressure, expel all air from the pipe. If hydrants or blowoffs are not available at high places, make the necessary taps at the points of highest elevation before testing, and insert plugs after the test has been completed.
5. Carefully examine all exposed pipes, fittings, valves, and hydrants during the test. Remove any cracked or defective pipes, fittings, valves, or hydrants discovered in consequence of this pressure test, and replace with sound material in the manner specified. Repeat the test until the results are satisfactory to the A/E.

5.2.13 DISINFECTION

A. During construction, take precautions to protect pipe interiors, fittings, and valves against contamination. When pipe laying is not in progress (e.g., at the end of the day's work), place watertight plugs in the ends of all pipe already in the trench; if water accumulates in the trench, leave the plugs in place until the trench is dry. Complete the joints of all pipe in the trench before stopping work for any reason.

B. If dirt or other foreign material that has gotten into a pipe will not, in the opinion of the A/E, be removed by flushing, clean the interior of the pipe, and swab with a disinfecting solution of 5% hypochlorite.

C. Make water flow from the existing distribution system or some other source approved by the A/E into the newly laid pipeline, and add chlorine to it. Feed water into the pipe, and chlorine into the water, at constant, measured rates so proportioned that the chlorine concentration in the water in the pipe is kept at a minimum of 50 mg/l available chlorine. To ensure that this concentration is maintained, measure the chlorine residual at regular intervals.

D. Table I shows how much chlorine is needed for each 100 feet of line for pipes of various diameters. A 1% chlorine solution may be prepared either with 1 pound of calcium hypochlorite for each 8.5 gallons of water or with sodium hypochlorite.
TABLE I

CHLORINE REQUIRED TO PRODUCE A 50 MG/L CONCENTRATION IN 100 FEET OF PIPE, BY DIAMETER

<table>
<thead>
<tr>
<th>Pipe Size (Inches)</th>
<th>100% Chlorine (Pounds)</th>
<th>1% Chlorine Solutions (Gallons)</th>
</tr>
</thead>
<tbody>
<tr>
<td>4</td>
<td>0.027</td>
<td>0.33</td>
</tr>
<tr>
<td>6</td>
<td>0.061</td>
<td>0.73</td>
</tr>
<tr>
<td>8</td>
<td>0.108</td>
<td>1.30</td>
</tr>
<tr>
<td>10</td>
<td>0.170</td>
<td>2.04</td>
</tr>
<tr>
<td>12</td>
<td>0.240</td>
<td>2.88</td>
</tr>
<tr>
<td>14</td>
<td>0.328</td>
<td>3.96</td>
</tr>
<tr>
<td>16</td>
<td>0.428</td>
<td>5.12</td>
</tr>
<tr>
<td>18</td>
<td>0.540</td>
<td>6.48</td>
</tr>
<tr>
<td>20</td>
<td>0.680</td>
<td>8.00</td>
</tr>
<tr>
<td>24</td>
<td>0.980</td>
<td>11.52</td>
</tr>
</tbody>
</table>

E. While the chlorine is being applied, manipulate valves so that the treatment dosage will not flow back into the line that is supplying the water. Continue the application of chlorine until the entire line being treated is filled with the chlorine solution. Then retain the chlorinated water in the line for at least 24 hours, during which time all valves and hydrants in the line being treated shall be operated so that appurtenances can also be disinfected. After 24 hours, the treated water shall have a chlorine concentration of at least 25 mg/l throughout the line.

F. After the applicable retention period, flush the heavily chlorinated water from the line until the chlorine concentration in the water leaving the main is no higher than that generally prevailing in the system, or less than 1 mg/l. Perform such flushing only at sites where there is adequate drainage.

G. The velocity of the water used to flush a line shall be at least 2.5 fps. The flow rates required to produce this velocity in various sizes of pipe are shown in Table II.
TABLE II
REQUIRED OPENINGS TO FLUSH PIPELINES
(40 PSI RESIDUAL PRESSURE)

REQUIRED OPENINGS TO FLUSH PIPELINES
(40 PSI RESIDUAL PRESSURE)

<table>
<thead>
<tr>
<th>Pipe Size (Inches)</th>
<th>Flow Required To Produce 2.5 fps Velocity (gpm)</th>
<th>Orifice Size (Inches)</th>
<th>Hydrant Outlet Nozzles Size</th>
<th>Number (Inches)</th>
</tr>
</thead>
<tbody>
<tr>
<td>4</td>
<td>100</td>
<td>15/16</td>
<td>1</td>
<td>2-1/2</td>
</tr>
<tr>
<td>6</td>
<td>220</td>
<td>1 - 3/8</td>
<td>1</td>
<td>2-1/2</td>
</tr>
<tr>
<td>8</td>
<td>390</td>
<td>1-7/8</td>
<td>1</td>
<td>2-1/2</td>
</tr>
<tr>
<td>10</td>
<td>610</td>
<td>2-5/16</td>
<td>1</td>
<td>2-1/2</td>
</tr>
<tr>
<td>12</td>
<td>880</td>
<td>2-13/16</td>
<td>1</td>
<td>2-1/2</td>
</tr>
<tr>
<td>14</td>
<td>1,200</td>
<td>3-1/4</td>
<td>2</td>
<td>2-1/2</td>
</tr>
<tr>
<td>16</td>
<td>1,565</td>
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<td>2</td>
<td>2-1/2</td>
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<tr>
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<td>1,980</td>
<td>4-3/16</td>
<td>2</td>
<td>2-1/2</td>
</tr>
<tr>
<td>20</td>
<td>2,440</td>
<td>---</td>
<td>2</td>
<td>2-1/2</td>
</tr>
<tr>
<td>24</td>
<td>3,470</td>
<td>---</td>
<td>2</td>
<td>2-1/2</td>
</tr>
</tbody>
</table>

H. Once a line has been flushed, test to make certain that the residual chlorine in the water is within acceptable limits.

I. It must be noted that flushing is no substitute for taking preventative measures before and during the laying of water lines. Certain contaminants--especially those in caked deposits--are difficult or even impossible to remove by flushing, no matter how high the velocity. Further-more, in pipes with diameters of 16 inches or more, it can be difficult to achieve even the minimum recommended flushing velocity of 2.5 fps.

5.2.14 BACTERIOLOGICAL TESTS

A. After a water line has undergone final flushing but before it is placed into service, collect a sample for bacteriological testing from the end of that line. In the case of extremely long lines, take additional samples if the A/E so directs.

B. Collect these samples in sterile bottles treated with sodium thiosulfate. Do not use a hose or fire hydrant to collect samples. One suggested sampling method is to install a standard corporation cock in the line with a copper tube gooseneck assembly; after the samples have been taken, the gooseneck assembly can be removed and retained for later use.

C. Take the samples collected to an approved laboratory to be tested for bacteriological quality in order to determine if they contain any coliform organisms. If the initial disinfection fails to produce satisfactory samples, repeat disinfection until satisfactory samples are obtained.

D. When the samples tested are found to be satisfactory, the water line may be placed in service.
5.2.15 **DISINFECTION PROCEDURE AFTER CUTTING INTO OR REPAIRING EXISTING LINES**

A. The procedures outlined above apply primarily to cases in which the lines are wholly or partially dewatered.

B. However, leaks or breaks that are repaired with clamping devices while the lines remain full of water under pressure present little danger of contamination and require no disinfection.

C. When an existing line is opened, whether by accident or design, the excavated area could be wet and contaminated because of the presence of sewers nearby. The danger of contamination from such pollution can be lessened if liberal quantities of hypochlorite are applied to the open trenches. It is better to use tablets for disinfection in such cases because they dissolve slowly and continue to release hypochlorite as water is pumped from the excavation site.

D. Where practical, treat the lines by the slug method in accordance with AWWA C651.

E. The following disinfection procedure is considered the minimum that may be used when existing lines are repaired:
   1. Swab the interior of all pipes and fittings (particularly couplings and tapping sleeves) that are to be used in repairing an existing line with a solution of 5% hypochlorite before installing them.
   2. The most practical means of removing contamination introduced into a line during repairs is to give the line a thorough flushing. If the locations of valves and hydrants make it possible, flushing in both directions is recommended. Start flushing as soon as repairs are completed, and continue until all discolored water is eliminated.

5.2.16 **CLEANUP**

A. After completing each section of water line, remove all debris and all construction materials from the work site. Then grade and smooth over the surface of both sides of the line. Leave the entire area clean and in a condition satisfactory to the A/E.
SECTION 6 - PAVEMENT REPLACEMENT

6.01 GENERAL

The CONTRACTOR shall be responsible for replacement of pavement removed or damaged by his operations. Pavement replacement shall be in accordance with this section of the specifications and in every case shall be equal to or better than the quality of pavement damaged or removed. The CONTRACTOR shall also be responsible for subsequent pavement failures during the warranty period, where such failures occur over or during the warranty period, where such failures occur over or adjacent to trenches or other excavations by the CONTRACTOR and result from insufficient compaction of the backfill.

6.02 PAVEMENT REMOVAL

Where existing paved streets, roads, parking lots, drives or sidewalks must be disturbed during construction of the project the CONTRACTOR shall take the necessary steps to minimize damage. Permanent type pavement shall be cut or sawed in a straight line before removal and care shall be taken during excavation to avoid damage to adjacent pavement. Where trucks or other heavy equipment must cross curbs or sidewalks, such areas shall be suitably protected.

6.03 PAVEMENT REPLACEMENT

Before trenching in paved areas the CONTRACTOR shall cut through the pavement in a straight line along the sides of the proposed trench so that the pavement may be removed and the trench may be dug without damage to the adjacent pavement. During construction suitable precautions shall be taken to protect the pavement edges and surfaces and minimize damage.

As soon as the pipe has been installed the trench shall be backfilled as specified in Section 4.10.

The permanent pavement patch shall not be made until the job is nearing completion in order to allow maximum time for any further settlement. The permanent pavement patch shall conform to the following schedule:

1. **Type “A”** - Principal highways, including traffic lanes, and turn lanes - eight inch (8") thick reinforced concrete slab over excavated areas plus two inches (2") of hot plant mix. (Hot Mix). If allowable by the Tennessee Department of Transportation, fourteen inches (14") of Binder may be used instead of the eight inch (8") reinforced concrete.

2. **Type “B”** - Parking areas, shoulders, turnouts and driveways with equivalent pavement - Minimum two inch (2") hot plant mix over 3 inch (3") binder.

3. **Type “C”** - Crushed stone driveways or roadways shall be eight inch (8") crushed stone base.

The hot mix and surface treatment applications shall be in accordance with standard specifications and recommended practices of the Tennessee Highway Department.
Pavement replacement shall extend a minimum of one foot (1′) beyond the trench line, and shall include replacement of all defective pavement resulting from the CONTRACTOR’S operations, regardless of whether caused by blasting, trenching, equipment operation, cave-in or other cause. Where the cut edge of pavement is less than one foot (1′) from the edge of the trench, or has been disturbed during construction, the CONTRACTOR shall cut through and remove existing pavement as required to permit a neat pavement patch. Irregular or uneven patches will not be permitted.

The CONTRACTOR shall be responsible for maintaining temporary patches during construction and shall promptly repair any defects. Upon completion of the work the paved surfaces shall be left in as good or better condition than before the start of construction.

Concrete driveways, sidewalks, curbs and gutters, etc., shall be of Class "A" Concrete of dimensions equivalent to original construction.

The type and nature of any pavement replacement shall at a minimum meet the standards and specifications of the governing authority.
SECTION 7 - TESTING AND ACCEPTANCE

7.01 **GENERAL**

Testing and acceptance of work shall be conducted as work proceeds and upon completion of the various work operations. Acceptance of the project shall involve a visual inspection and/or a leakage test. The procedures shall be as outlined hereinafter. The work will not be accepted until the visual inspection and/or the leakage test results are satisfactory.

7.02 **CLEANING**

Upon completion of cleaning of any line or manhole the **ENGINEER** shall make a visual inspection to verify the quality of workmanship. Any defects such as grease or roots shall be removed by means of further cleaning operations until the line or manhole is in a condition satisfactory to the **ENGINEER**.

7.03 **MANHOLES**

Once all manholes have been constructed or repaired and proper curing time for materials has elapsed, a vacuum test shall be conducted on the manholes. The test shall be conducted by the **CONTRACTOR** in coordination with the **ENGINEER** as specified hereinafter.

The test shall be considered acceptable when the vacuum remains at 10” of Mercury (Hg), or drops no lower than to 9” Hg within the time period specified below.

Manholes 4-foot in diameter shall be required to sustain a vacuum with no more than 1” Hg drop according to the following Schedule.

<table>
<thead>
<tr>
<th>Manhole Depth</th>
<th>Time to Drop 1” Hg</th>
</tr>
</thead>
<tbody>
<tr>
<td>4-10 ft</td>
<td>75 seconds</td>
</tr>
<tr>
<td>10-15 ft</td>
<td>90 seconds</td>
</tr>
<tr>
<td>15-25 ft</td>
<td>105 seconds</td>
</tr>
</tbody>
</table>

If the manhole fails the test, necessary repairs shall be made and the vacuum test repeated until the manhole passes the test. If the manhole joint mastic or gasket is displaced during the vacuum test, the manhole shall be disassembled, the seal replaced, and the manhole retested.

Before testing, all pipes and other openings into the manhole shall be suitably plugged in such a manner as to prevent displacement of the plugs while a vacuum is pulled. Installation and operation of the vacuum equipment shall be in accordance with the equipment specifications and instructions provided by the manufacturer. The test head shall be placed in the cone section of the manhole.

Before final acceptance, Smyrna will televise the interior of the sanitary sewer main to help assure compliance with the design, material, workmanship and record drawings. Any defects noted in this inspection shall be corrected by the contractor.

All manholes that are tested must be tested after castings are installed even if tested previously without castings. If the source of the problem is something other than that originally specified for the manhole, then that problem shall be corrected in accordance with the methods prescribed in these specifications, and the **CONTRACTOR** shall receive additional payment for this work based on the method of repair used. If, however, the source of the problem is related to the
original problem, then it shall be corrected by the CONTRACTOR as prescribed in these specifications at no additional cost to the OWNER.

7.04  GRAVITY SEWER LINES & SEWER LINE REPLACEMENT

(1)  **GENERAL** - Upon completion of construction the CONTRACTOR shall remove all sand, dirt, brick and other foreign materials from the sewers and shall conduct his own inspection to locate any defects and determine when the sewers are ready for final inspection, testing and acceptance by the ENGINEER.

After all apparent defects have been corrected, the CONTRACTOR shall notify the ENGINEER and request a final inspection.

No final inspection will be scheduled by the ENGINEERS until the CONTRACTOR advises that he has conducted his own inspection and believes the project to be ready for such final inspection. Should the ENGINEER begin a final inspection at the request of the CONTRACTOR and find that the sewers have not been cleaned or defects have not been corrected, the inspection will be terminated and will not be rescheduled until the CONTRACTOR again advises that the project is ready for inspection.

Acceptance of the project shall involve visual inspection leakage test and a deflection test. The procedures shall be as outlined hereinafter. The work will not be accepted until the visual inspection, leakage test, deflection, and test results are satisfactory. A deflection test will be required for PVC sewer lines running a full manhole to manhole length. Deflection test shall be by pulling a 9-arm mandrel sized at 95% of the internal diameter through the sewer. Test shall be performed after the sewer has been backfilled for at least 24 hours. PVC sewer lines failing mandrel test must be relayed.

(2)  **VISUAL INSPECTION** - The ENGINEER will, as a part of the final inspection, make the necessary visual inspections to verify the quality of workmanship.

Such inspections shall include examination of manholes, "lamping" or "flashing" sewer lines and observation of clean-up, pavement replacement, etc.

Any defects such as misalignment of sewers, visible leaks, obstructions, cracked or broken pipe, or failure to restore the surface to a satisfactory condition must be corrected to the ENGINEER’S satisfaction before acceptance. Any sags, humps, bends or other evidence of misalignment regarding of type of pipe shall be cause for rejection.

(3) **LEAKAGE TESTS** - After completion of sewer construction and following the visual inspection a low pressure air test shall be performed on all sewers to determine leakage. The CONTRACTOR will furnish all equipment and facilities and all personnel for conducting the test. The test shall be observed by a representative of the ENGINEER.

The air test will be made after all services have been installed and backfilling has been completed and compacted.
All ties and end of sewer services shall be plugged with flexible joint plugs or caps securely fastened to withstand the internal test pressures.

Such plug or cap shall be readily removable, and their removal shall provide a socket suitable for making a flexible jointed lateral connection or extension.

Air shall be slowly supplied to the plugged pipe installation until the internal air pressure reaches 4.0 pounds per square inch greater than the average back pressure of any ground water that may submerge the pipe. At least two (2) minutes shall be allowed for temperature stabilization.

The requirements of this specification shall be considered satisfied if the time required in seconds for the pressure to decrease from 3.5 to 2.5 pounds per square inch greater than the average back pressure of any ground water that may submerge the pipe is not less than what is shown on the "Allowable Time Table" listed below.

If the pipe installation fails to meet these requirements, the CONTRACTOR shall determine at his own expense the source or sources of leakage; and he shall repair or replace all defective materials or workmanship.

Procedures for Conducting Acceptance Air Tests shall be as follows:

(a) Clean Pipe to be tested.

(b) Plug all pipe outlets with suitable test plugs. Brace each plug securely.

(c) Increase gauge pressure in the test by the amount of ground water pressure at the crown of the pipe.

(d) Add air slowly to the portion of the pipe installation under test until the internal air pressure is raised to 4.0 pounds per square inch greater than the average back pressure above the crown of the pipe.

(e) After the above internal pressure is obtained, allow at least two (2) minutes for air temperature to stabilize adding only the amount of air required to maintain pressure.

(f) After the two minute period, disconnect air supply.

(g) When pressure decreases to 3.5 psig, start stopwatch. Determine the time in seconds that is required for the internal air pressure to reach 2.5 psig. This time interval should then be compared with the time shown in the "Allowable Time Table". If the time is more than that shown in the table the test will be assumed to be acceptable.
<table>
<thead>
<tr>
<th>Pipe Size (Inches)</th>
<th>Time, T (sec/100 ft.)</th>
<th>Allowable Air Loss, Q (ft³/min)</th>
</tr>
</thead>
<tbody>
<tr>
<td>6</td>
<td>42</td>
<td>2.0</td>
</tr>
<tr>
<td>8</td>
<td>72</td>
<td>2.0</td>
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<td>10</td>
<td>90</td>
<td>2.5</td>
</tr>
<tr>
<td>12</td>
<td>108</td>
<td>3.0</td>
</tr>
<tr>
<td>15</td>
<td>126</td>
<td>4.0</td>
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<tr>
<td>18</td>
<td>144</td>
<td>5.0</td>
</tr>
<tr>
<td>21</td>
<td>180</td>
<td>5.5</td>
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<tr>
<td>24</td>
<td>216</td>
<td>6.0</td>
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<tr>
<td>27</td>
<td>252</td>
<td>6.5</td>
</tr>
<tr>
<td>30</td>
<td>288</td>
<td>7.0</td>
</tr>
</tbody>
</table>

Plugs used to close the sewer pipe for the air test must be securely braced to prevent the unintentional release of a plug that can become a high velocity projectile. Gauges, air piping manifolds and valves shall be located at the top of the ground. No one shall be permitted to enter a manhole where a plugged pipe is under pressure. Four pounds (4#) air pressure develops a force against the plug in a 12-inch (12") diameter pipe of approximately 450 pounds (450#).

A safety release device set to release at ten pounds (10#) per square inch is to be provided between the air supply and the sewer under test.

In addition to the leakage tests above, an infiltration leakage test will be made to insure compliance with the infiltration limitations. Infiltration shall not exceed 25 GPD per inch diameter per mile of sewer and in no case shall it exceed 750 GPD per mile of sewer. The infiltration test shall be made a maximum ground water table.

The **CONTRACTOR** shall furnish all labor, tools, equipment and materials for the test. The test must be scheduled at a time acceptable to the **ENGINEER** and shall be witnessed by his representative.

### 7.05 FORCE MAINS

1. **GENERAL** - Upon completion of the construction work under this contract all force mains shall be subjected to the necessary pressure and leakage tests. In the event the pressure or leakage test is unsatisfactory corrective measures shall be taken and the tests repeated until satisfactory results are obtained. Force mains shall be tested and accepted only in accordance with AWWA C-600 and these Specifications.

2. **PRESSURE AND LEAKAGE TESTS** - All lines shall be subjected to a hydrostatic pressure of 200 psi for a period of two hours, and any defective work revealed by the test shall be repaired or replaced by the **CONTRACTOR**.
SECTION 8 - WARRANTY AND MAINTENANCE OBLIGATIONS

8.01 WARRANTY

The work to be performed under this contract shall be guaranteed against defects in materials or workmanship for a period of one year following the date of formal acceptance of the project. In the event defects in materials or workmanship should appear, the CONTRACTOR shall promptly make the necessary corrections. When the defects are not of an emergency nature, the CONTRACTOR will be notified and will be given a period of two (2) weeks in which to make the necessary corrections. Should the defects be of an emergency nature that in the opinion of the OWNER or the ENGINEER requires immediate correction, the CONTRACTOR will be notified and requested to make the necessary repairs immediately. Should this be impractical or if the CONTRACTOR should fail to respond to the request for corrective action within the specified period, the OWNER may proceed to have the defects corrected and shall bill the CONTRACTOR for all charges in connection therewith including labor, materials and equipment rental. Such charges may be deducted from amounts due the CONTRACTOR if any of the CONTRACTOR’S money has been withheld. In the event the CONTRACTOR fails, refuses or neglects to pay the OWNER, the surety shall be liable for such charges.

8.02 MAINTENANCE OBLIGATION

The CONTRACTOR shall be fully responsible for maintenance of any and all portions of the work that he performs under this contract for a period of ninety (90) days. This maintenance obligation shall begin upon formal acceptance of the project and is intended to place a limit upon the CONTRACTOR’S responsibility for normal maintenance required for the routine operation of the system. This ninety (90) day obligation shall not be construed as relieving the CONTRACTOR of the responsibility for maintenance or repair work resulting from defective materials or workmanship.
SECTION 9 - SPECIAL CONDITIONS

9.01 GENERAL

The CONTRACTOR’S attention is called for the special conditions indicated on the plans and described in this Section of the specifications. Special conditions include construction on highway rights-of-way, and construction in the vicinity of existing utilities. The OWNER will make application to the Highway Department for the necessary permits and the utilities involved will be notified of the proposed construction. The plans and specifications reflect the type of construction that is anticipated in the various locations requiring special attention but it shall be the responsibility of the CONTRACTOR to contact the various agencies including the State Highway Department, the gas company, telephone company and other utilities involved when working in areas where they will be concerned, and for coordinating construction with their requirements in such a way to avoid conflicts, damage or interruptions in service.

9.02 WORK ON STATE HIGHWAY DEPARTMENT RIGHTS-OF-WAY

Plans and specifications for this project will be submitted to the State Highway Department and an application will be made for a permit to construct and/or repair sewers on Highway rights-of-way. In the event that a bond is required, said bond will be provided by the CONTRACTOR and will be required to conform to the conditions of the permit and bond.

Where it is necessary to make cuts in pavement along or across U.S. Highways, replacement of the pavement shall conform to Section 6.03 of these specifications.

When working in or near lanes of traffic, the CONTRACTOR shall provide warning signals or flagmen as required by the Highway Department and shall prosecute the work in such a way as to cause a minimum of inconvenience to the traveling public.

9.03 DISPOSAL OF WASTE MATERIALS

The CONTRACTOR shall dispose all waste debris generated as a result of cleaning operations and line repair at an area approved by the OWNER and ENGINEER.

9.04 SEPARATION OF WATER MAINS AND SEWERS

a. General - The following factors should be considered in providing adequate separation:

1. Materials and type of joints for water and sewer pipes;
2. Soil conditions;
3. Service and branch connections into the water main and sewer lines;
4. Compensation variations in the horizontal and vertical separations;
5. Space for repair and alterations of water and sewer pipes;
6. Off-setting of pipes around manholes.

b. Parallel Installation
1. **Normal conditions** - Water mains shall be laid at least ten feet (10') horizontally from any sanitary sewer, storm sewer or sewer manhole, whenever possible. The distance shall be measured edge-to-edge.

2. **Unusual conditions** - When local conditions prevent a horizontal separation of ten feet (10’), a water main may be laid closer to a storm or sanitary sewer provided that:
   
   (i) The bottom of the water main is at least eighteen inches (18”) above the top of the sewer;

   (ii) Where this vertical separation cannot be obtained, the sewer shall be constructed of materials and with joints that are equivalent to water main standards of construction and shall be pressure tested to assure watertightness prior to backfilling.

**c. Crossing**

1. **Normal conditions** - Water mains crossing house sewers, storm sewers or sanitary sewers shall be laid to provide a separation of at least eighteen inches (18”) between the bottom of the water main and the top of the sewer, whenever possible.

2. **Unusual conditions** - When local conditions prevent a vertical separation as described in Section 9.04-b-2 above, the following construction shall be used:
   
   (i) Sewers passing over or under water mains should be constructed of the materials described in Section 9.04-(b)-2-ii.

   (ii) Water mains passing under sewers shall, in addition, be protected by providing:

   (I) A vertical separation of at least eighteen inches (18”) between the bottom of the sewer and the top of the water main;

   (II) Adequate structural support for the sewers to prevent excessive deflection of joints and settling on and breaking the water mains;

   (III) That the length of water pipe be centered at the point of crossing so that the joints will be equidistant and as far as possible from the sewer;

   (IV) Both the sewer and the water main shall be constructed of iron pipe and tested in accordance with Section 7.

**d. Sewer manholes** - No water pipe shall pass through or come into contact with any part of a sewer or sewer manhole.

**9.05 REPAIR OF MANHOLES**

The work consists of repairing leaks found during high groundwater table conditions. The **CONTRACTOR** shall take precautions in insuring that all defects are repaired when the manholes are first scheduled for work. Should the manhole
be found to be leaking anytime during the warranty period from a leak type repaired by the CONTRACTOR, the manhole will be repaired at no additional cost to the OWNER.

9.06 **ABANDON MANHOLE**

Wherever shown, the CONTRACTOR shall remove existing casting and deliver such to City's inventory. In areas subject to traffic, manhole shall be filled with crushed stone, and repair pavement. In yards and fields CONTRACTOR shall remove top two feet (2') of manhole, fill manhole with approved material and seed disturbed area. CONTRACTOR to be responsible for settlement. Inlet and out lines to be plugged with concrete as directed by ENGINEER.

9.07 **MANHOLE CONNECTIONS**

CONTRACTOR shall construct all manhole connections as shown on detail sheet of these specifications for the various methods (A, B, & C). For Method D, the CONTRACTOR shall make connections to existing manholes as shown on plans, reroute, reform and rebuild inverts, plug and seal existing lines as directed, repair all defects within manhole, plug and seal all leaks.

9.08 **RECONDITION MANHOLES**

Contract shall plug and seal existing lines as shown on Plans, reconstruct or recondition invert as required, repair all defects, plug and seal all leaks and plaster manhole in accordance with approved system.

9.09 **SLOPE PROTECTION AND EROSION CONTROL**

A. **General**

This section shall consist of temporary control measures as shown in the Plans or directed by the ENGINEER during the life of the Contract to control erosion and water pollution through the use of berms, dikes, dams, sediment basins, fiber mats, netting, mulches, grasses, slope drains, temporary silt fences, and other control devices.

The temporary pollution control provisions contained herein shall be coordinated with the permanent erosion control features to assure economical effective and continuous erosion control throughout the construction and post-construction period.

B. **Materials**

1. **Temporary Berms:**

   A temporary berm is constructed of compacted soil, with or without a shallow ditch, at the top of fill slopes or transverse to centerline on fills.

   These berms are used temporarily at the top of newly constructed slopes to prevent excessive erosion until permanent controls are installed or slopes stabilized.

2. **Temporary Slope Drains**

   A temporary slope drain is a facility consisting of stone gutters, fiber mats,
plastic sheets, concrete or asphalt gutters, half-round pipe, metal pipe, plastic pipe, sod or other material acceptable to the ENGINEER that may be used to carry water down slopes to reduce erosion.

3. **Sediment Structures**

Sediment basins, ponds and traps are prepared storage areas constructed to trap and store sediment from erodible areas in order to protect properties and stream channels below the construction areas from excessive siltation.

4. **Check Dams**

a. Check dams are barriers composed of logs and poles, large stones or other materials placed across a natural or constructed drainway.

b. Stone check dams shall not be utilized where the drainage area exceeds fifty (50) acres. Log and pole structures shall not be used where the drainage area exceeds five (5) acres.

5. **Temporary Seeding and Mulching**

Temporary seeding and mulching are measures consisting of seeding, mulching, fertilizing, and matting utilized to reduce erosion. All cut and fill slopes including waste sites and borrow pits shall be seeded when and where necessary to eliminate erosion.

6. **Brush Barriers**

a. Brush barriers shall consist of brush, tree trimmings, shrubs, plants and other approved refuse from the clearing and grubbing operations.

b. Brush barriers are placed on natural ground at the bottom of all slopes where the most likely erodible areas are located to restrain sedimentation particles.

7. **Baled Hay or Straw Checks**

a. Baled hay or straw erosion checks shall not be allowed.

8. **Temporary Silt Fences**

Silt fences are temporary measures utilizing woven wire or other approved material attached to posts with filter cloth composed of burlap, plastic filter fabric, etc., attached to the upstream side of the fence to retain the suspended silt particles in the run-off water.

C. **EXECUTION**

1. **Project Review**

Prior to the Pre-Construction Meeting the CONTRACTOR shall meet with the ENGINEER and go over in detail the expected problem areas in regard to the erosion control work. Different solutions should be discussed so that the best method might be determined. It is the basic responsibility of the
CONTRACTOR to develop an erosion control plan acceptable to the ENGINEER.

2. **Pre-Construction Meeting**

At the Pre-Construction Meeting the CONTRACTOR shall submit for acceptable his schedule for accomplishment of temporary and permanent erosion control work, as are applicable for clearing and grubbing, grading, bridges and other structures at watercourses, construction and paving. He shall also submit for acceptance his proposed method of erosion control on haul roads and borrow pits and his plan for disposal of waste materials. No work shall be started until the erosion control schedules and methods of operations have been accepted by the ENGINEER.

3. **Construction Requirements**

a. The ENGINEER has the authority to limit the surface area of erodible earth material exposed by clearing and grubbing, the surface of erodible earth material exposed by excavation, borrow and fill operations and to direct the CONTRACTOR to provide immediate permanent or temporary pollution control measures to prevent contamination of adjacent streams or other watercourses, lakes, ponds, or other water impoundment.

Such work may involve the construction of temporary berms, dikes, dams, sediment basins, slope drains, and use of mulches, mats, seeding or other control devices or methods as necessary to control erosion. Cut and fill slopes shall be seeded and mulched as the excavation proceeds to the extent directed by the ENGINEER.

b. The CONTRACTOR shall be required to incorporate all permanent erosion control features into the project at the earliest practicable time as outlined in his accepted schedule. Temporary pollution control measures shall not be used to correct conditions that develop during construction that were not foreseen during the design stage; that are needed prior to installation of permanent pollution control features; or that are needed temporarily to control erosion that develops during normal construction practices, but are not associated with permanent control features on the project.

c. Where erosion is likely to be a problem, clearing and grubbing operations should be so scheduled and performed that grading operations and permanent erosion control features can follow immediately thereafter if the project conditions permit; otherwise erosion control measures may be required between successive construction stages. Under no conditions shall the surface area of erodible earth material exposed at one time by clearing and grubbing exceed 750,000 square feet without the approval of the ENGINEER.

d. The ENGINEER will limit the area of excavation, borrow and embankment operations in progress commensurate with the CONTRACTOR'S capability and progress in keeping the finish grading, mulching, seeding and other such permanent pollution control measures current in accordance with the accepted
schedule. Should seasonal limitations make such coordination unrealistic, temporary erosion control measures shall be taken immediately to the extent feasible and justified.

e. Under no conditions shall the amount of surface area or erodible earth material exposed at one time by excavation or fill within the project area exceed 750,000 square feet without prior approval by the ENGINEER.

f. The ENGINEER may increase or decrease the amount of surface area of erodible earth material to be exposed at one time by clearing and grubbing, excavation, borrow and fill operations as determined by his analysis of project conditions.

g. In the event of conflict between these requirements and pollution control laws, rules or regulations, or other Federal, State or Local agencies, the more restrictive laws, rules or regulations shall apply.

4. Construction of Structures

a. Temporary Berms

A temporary berm shall be constructed of compacted soil with a minimum width of twenty four inches (24") at the top and a minimum height of twelve inches (12") with or without a shallow ditch, constructed at the top of fill slopes or transverse to centerline on fills. Temporary berms shall be graded so as to drain to a compacted outlet at a slope drain. The area adjacent to the temporary berm in the vicinity of the slope drain must be properly graded to enable this inlet to function efficiently and with minimum ponding in this area. All transverse berms required on the downstream side of a slope drain shall extend across the grade to the highest point with an approximate ten degree (10°) angle perpendicular to the centerline. The top width of these berms may be wider and the side slope flatter on transverse berms to allow equipment to pass over these berms with minimal disruptions. When practical and until final roadway elevations are approached, embankments should be constructed with a gradual slope to one side of the embankment to permit the placement of temporary berms and slope drains on only one side of the embankment.

b. Temporary Slope Drains

1. Temporary slope drains shall consist of stone gutters, fiber mats, plastic sheets, concrete or asphalt gutters, half-round pipe, metal pipe, plastic pipe, flexible rubber, or other materials which can be used as temporary measures to carry water accumulating in the cuts and on the fills down the slopes prior to installation of permanent facilities or growth of adequate ground cover on the slopes.

2. Fiber matting and plastic sheeting shall not be used on slopes steeper than 4:1 except for short distances of twenty feet (20') or less.
3. All temporary slope drains shall be adequately anchored to the slope to prevent disruption by the force of the water flowing in the drains. The base for temporary slope drains shall be compacted and concavely formed to channel the water or hold the slope drain in place.

The inlet end shall be properly constructed to channel water into the temporary slope drain. Energy dissipators, sediment basins or other approved devices shall be constructed at the outlet end of the slope drains to reduce erosion downstream. An ideal dissipator would be dumped rock or a small sediment basin that would slow the water as well as pick up some sediment. All temporary slope drains shall be removed when no longer necessary and the site restored to match the surroundings.

c. **Sediment Structures**

1. Sediment structures shall be utilized to control sediment at the foot of embankments where slope drains outlet; at the bottom as well as in the ditch lines atop waste sites; in the ditch lines or borrow pits. Sediment structures may be used in most drainage situations to prevent excessive siltation of pipe structures. All sediment structures shall be at least twice as long as they are wide.

2. When use of temporary sediment structures is to be discontinued, all sediment accumulation shall be removed and all excavation backfilled and properly compacted. The existing ground shall be restored to its natural or intended condition.

d. **Check Dams**

1. Check dams shall be utilized to retard stream flow and catch small sediment loads. Materials utilized to construct check dams are varied and should be clearly illustrated or explained in the **CONTRACTOR’S** erosion control plan.

2. All check dams shall be keyed into the sides and bottom of the channel a minimum depth of two feet (2’). A design is not needed for check dams but some typical designs are shown in the standard plans.

3. Stone check dams should generally not be utilized where the drainage area exceeds fifty (50) acres. Log and pole structures should generally not be used where the drainage area exceeds five (5) acres.

e. **Temporary Seeding and Mulching**

Seeding and mulching shall be performed in accordance with Section 02828-Miscellaneous Seeding.

f. **Brush Barriers**

Brush barriers shall consist of brush, tree trimmings, shrubs, plants
and other approved refuse from the clearing and grubbing operations. The brush barriers shall be constructed approximately parallel to original ground contour. The brush barrier shall be compressed to an approximate height of three (3) to five (5) feet and approximate width of five (5) to ten (10) feet. The embankment shall not be supported by the construction of brush barriers.

h. **Temporary Silt Fences**

1. Temporary silt fences shall be placed on the natural ground, at the bottom of fill slopes, in ditches, or other areas where siltation is a problem. Silt fences are constructed of wire mesh fence with a covering of burlap or some other suitable material on the upper grade side of the fence and anchored into the soil.

2. The CONTRACTOR shall be required to maintain the silt fence in a satisfactory condition for the duration of the project or until its removal is requested by the ENGINEER. The silt accumulation at the fence may be left in place and seeded, removed, etc., as directed by the ENGINEER. The silt fence becomes the property of the CONTRACTOR whenever the fence is removed.

D. **MAINTENANCE**

a. The temporary erosion control features installed by the CONTRACTOR shall be acceptably maintained by the CONTRACTOR until no longer needed or permanent erosion control methods are installed. Any materials removed shall become the property of the CONTRACTOR.

b. In the event that temporary erosion and pollution control measures are required due to the CONTRACTOR'S negligence, carelessness or failure to install permanent controls as a part of work as scheduled, and are ordered by the ENGINEER, such work shall be performed by the CONTRACTOR at his own expense.

c. Where the work to be performed is not attributed to the CONTRACTOR'S negligence, carelessness, or failure to install permanent controls and falls within the specifications for a work item that has a contract price, the units of work shall be paid for at the proper contract prices.

E. **EROSION CONTROL OUTSIDE PROJECT AREA**

Temporary pollution control shall include construction work outside the project area where such work is necessary as a result of construction such as borrow pit operations, haul roads and equipment storage sites. Bid price in such cases shall include all necessary clearing and grubbing, construction incidentals, maintenance and site restoration when no longer needed.
F. **MEASUREMENT AND PAYMENT**

No separate Measurement and Payment will be made for this work. It will be considered a subsidiary obligation of the CONTRACTOR under other bid items to which it relates.

9.10 **VIDEO**

Prior to construction, CONTRACTOR shall color video tape the entire project including the route of the line construction, all easement areas, the full width of all rights-of-ways, and all service line areas. The CONTRACTOR shall identify the line designation and station number, all natural landmarks, the street address of the area in view and all potential areas, structures, fences, trees, etc., subject to potential disturbance. The CONTRACTOR shall provide the owner with two (2) copies of the video with audio comments.

9.11 **FINAL CLEAN-UP AND RESTORATION**

In all areas damaged or disturbed by CONTRACTOR’S operations where established ground cover was present before beginning of construction, CONTRACTOR shall be responsible for restoring this ground cover after completion of construction. (Unless noted otherwise on drawings). In areas of established lawns, CONTRACTOR will be required to: separate and preserve best of excavated material or, if no acceptable material has been excavated, haul in an acceptable material for use in making top six-inches (6”) of finished grade. No rock will be permitted in this top six-inches (6”) of finished grade for established lawns. All areas seeded shall be graded smooth prior to seeding and CONTRACTOR shall be responsible for maintenance of this smooth finished grade until grass growth is established.

After Designated areas have been carefully hand graded, soil shall be prepared for seeding. Where necessary, CONTRACTOR will sod slopes and embankments and remaining areas may be seeded.

A well made lawn is desired, and CONTRACTOR will be responsible for any necessary regrading or reseeding required to produce an acceptable grass as cover. The seed shall be the same type of grass existing before construction.

The soil shall be fertilized with a commercial fertilizer of a grade and at a rate recommended by vendor of seed.

All seeded areas shall be covered with clean straw uniformly distributed to an approved density.
SECTION 10 - MEASUREMENT AND PAYMENT

10.01 GENERAL

The CONTRACTOR shall furnish all labor, tools, equipment and materials to construct the proposed improvements complete as shown on the drawings and described in the specifications. The work shall be measured for payment in accordance with applicable provisions of these specifications and payment shall be made on the basis of the unit prices or lump sum prices bid. The sum of the payments for eligible pay items contained in the proposal from shall be the compensation to be paid for the completed project; provided however, that changes in the work covered by written change orders, properly executed may result in additions or deductions from the contract price.

The CONTRACTOR'S attention is called to the fact that although the pay items shown shall be the basis for establishing the contract price, the pay items do not necessarily reflect the total amount of work to be performed. The cost of incidental work such as clearing and grubbing, trenching, backfilling, testing, etc., which is necessary, but which is not specifically listed as one of the pay items, shall be included in the prices bid for the eligible pay items to which the incidental work is most closely related.

10.02 SEWER PIPE

a. Measurement - Sewer pipe shall be measured for payment by horizontal measurements or station distances along the sewer lines from transition in type of pipe, center of manholes, or center of fittings without deduction for space occupied by manholes or fittings. Sewer size shall be based on the nominal pipe diameter indicated for the respective locations.

Measurement for establishing cut classification shall be the vertical distance from undisturbed ground elevation to the invert of the sewer as determined by the ENGINEERS plans.

b. Payment - Sewer pipe shall be paid for on the basis of the respective unit prices bid per linear foot for pipe of the various sizes, materials and cut classifications.

Payment for sewer pipe shall constitute compensation in full for furnishing all labor, tools, equipment and materials and installing the sewer complete, including incidental work such as location and protection of existing utilities, clearing, excavation (including rock), dewatering trenches, bedding with crushed stone, crushed stone backfill up to a point 12" above pipe, crushed stone backfill in all roads, drives or areas subject to traffic, disposal of surplus excavated material, seeding, sodding or sprigging, cleaning, inspection and testing.

The cost of furnishing and installing adapters for transitions between pipe materials if required, shall also be included in the bid prices for sewer pipe inasmuch as no separate payment will be made.

Crushed stone and pavement replacement are covered in section 10.09 and 10.10.
10.03 SERVICE PIPE

a. **Measurement** - Service pipe shall be measured for payment by horizontal measurements along the service lines from center of fittings to end of service without deduction for space occupied by fittings.

No classification of cut depth shall be made for service lines.

b. **Payment** - Service pipe shall be paid for on the basis of the respective unit prices bid per linear foot for pipe as provided in the bid form.

Payment for service pipe shall constitute compensation in full for furnishing all labor, tools, equipment and materials and installing the service pipe complete, including incidental work such as location and protection of existing utilities, clearing, excavation, dewatering trenches, bedding with crushed stone, crushed stone backfill up to a point twelve inches (12") above pipe, crushed stone backfill in all areas subject to traffic, disposal of surplus excavated material, seeding, sodding or sprigging, cleaning, inspection and testing. The cost of furnishing and installing adapters for transition from one material or joint type to another shall also be included in the service line bid price inasmuch as no separate payment will be made.

10.04 SEWER PIPE FITTING

a. **Measurement** - Bends, reducers and wyes or tees shall be measured by actual count of each of the various sizes.

b. **Payment** - Payment for fittings shall be made on the basis of the unit price bid for each and shall reflect cost of fittings over and above cost of sewer.

10.05 MANHOLES

a. **Measurement** - Manholes shall be measured by actual count. Manhole depth shall be measured vertically from the invert at the center of the manhole to the top of the casting or cover.

Measurement of watertight manhole covers shall be by actual count of such covers actually installed.

b. **Payment** - Payment for manholes shall be made on the basis of the unit prices bid for each and shall constitute payment in full for furnishing all materials and constructing the manholes complete, including excavation, concrete, brick, plastering, castings, and other incidentals, with all manholes being considered as standard manholes. For manhole depth greater than 6'-0", extra payment shall be made per foot for the extra depth over 6'-0". Extra Payment for drop manholes shall be made per vertical foot as shown on Standard Detail (SD-3).

Payment for watertight manhole covers shall be made on the basis of the unit prices bid for each such cover, and shall constitute payment in full for the extra cost of furnishing and installing such covers over and above the cost of conventional covers as specified. Crushed stone and pavement replacement are covered in Sections 11.10 and 11.09 respectively.

10.06 ROCK EXCAVATION

a. **Measurement** - No measurement required.
b. **Payment** - No compensation shall be made for rock excavation. The cost of such rock excavation shall be included in the various unit items. Included in the Bid Proposal.

10.07 **CLASS A CONCRETE**

a. **Measurement** - Concrete work will be measured by calculating the actual volumes in structures or from invoice records, whichever is appropriate, to the nearest 0.1 cubic yard.

b. **Payment** - Payment for Class A concrete shall be made on the basis of the unit price bid per cubic yard and shall constitute full compensation for concrete, reinforcement, forms, anchor bolts, nuts, rods, excavation (except rock), backfilling, and other incidentals required to complete the work.

10.08 **CLASS B CONCRETE**

a. **Measurement** - Class B concrete used in bracing pipe and fittings shall be measured for payment on the basis of the theoretical quantities required to provide the desired bearing area with a trench of the desired dimensions. The pay quantities for braces behind typical fittings shall be as follows:

**PAY QUANTITIES FOR THRUST BLOCKS - CF CLASS B CONCRETE**

<table>
<thead>
<tr>
<th>PIPE DIA.</th>
<th>TEE</th>
<th>90°</th>
<th>45°</th>
<th>22-1/2°</th>
<th>DEAD END</th>
</tr>
</thead>
<tbody>
<tr>
<td>6&quot;</td>
<td>3.0</td>
<td>4.0</td>
<td>2.3</td>
<td>2.3</td>
<td>11.3</td>
</tr>
<tr>
<td>8&quot;</td>
<td>5.4</td>
<td>8.1</td>
<td>4.3</td>
<td>3.3</td>
<td>12.5</td>
</tr>
<tr>
<td>10&quot;</td>
<td>9.7</td>
<td>14.0</td>
<td>7.0</td>
<td>3.5</td>
<td>13.7</td>
</tr>
<tr>
<td>12&quot;</td>
<td>15.0</td>
<td>20.0</td>
<td>11.2</td>
<td>6.3</td>
<td>15.0</td>
</tr>
<tr>
<td>16&quot;</td>
<td>28.6</td>
<td>41.3</td>
<td>22.9</td>
<td>11.8</td>
<td>17.7</td>
</tr>
</tbody>
</table>

In the event the type of soil is such that the bearing area must be increased an appropriate adjustment will be made in the pay quantities, the adjustment being equal to the percentage adjustment in the bearing area required. For concrete used in over bends in the pipe line where no specified dimensions are shown for the thrust block, the measurement will be based on the actual quantity of concrete that the **ENGINEER** directs the **CONTRACTOR** to use.

Class B concrete used in cradling or encasement of sewers will be measured by computing the theoretical volume of concrete required within a ditch having a width equal to the nominal pipe diameter plus eighteen inches (18”). The length shall be the actual length of such concrete installed at the **ENGINEERS** direction. The depth shall be such as to extend from the spring line of the pipe to a point four inches (4”) above the pipe to a point six inches (6") below the bottom for encasement.

Measurement for Class B concrete used in pads, low piers, or blocks shall be based on the theoretical volume required for the dimensions of the structure as shown on the plans or as directed by the **ENGINEER**.

**Payment** - Payment for Class B concrete shall be made on the basis of the unit price bid per cubic yard, and shall constitute full compensation for excavation.
(except rock), forming, furnishing and placing the concrete, and other incidental work required to complete the work. No separate payment will be made for Class B concrete included in manholes, drop pipes, service risers, or other structures where the price of such concrete is included in the unit price of lump sum price bid for the item.

10.09 **PAVEMENT REPLACEMENT**

a. **Measurement** - Measurement for pavement replacement shall be equal to the length of the pavement cut multiplied by the width of pavement actually replaced with a strip having a maximum width equal to the nominal pipe diameter plus 3'-6" centered over the pipe line.

Around manholes, measurement of pavement replacement for payment shall be limited to an area 8' x 8', with appropriate deduction for pavement measured over trenches.

b. **Payment** - Payment for pavement replacement shall be made on the basis of the unit prices bid for various classifications of pavement as indicated in the proposal form. Such payment shall constitute full compensation for furnishing all labor, materials, and equipment and replacing the damaged pavement, including the crushed stone base as required. The **CONTRACTOR** is advised that although the limits of payment shall be as described under paragraph a, above, he shall be responsible for replacing all pavement damaged during construction, so that the paved area is left in a condition as good as or better than before the start of construction.

Payment for pavement replacement shall also include compensation for providing temporary pavement patches as required by the specifications and for maintaining the patches until such time as the permanent pavement is placed inasmuch as no separate payment will be made for this work.

10.10 **CRUSHED STONE**

a. **Measurement** - Measurement of crushed stone for payment shall be based on weight. In all cases delivery tickets shall be furnished to the **ENGINEER** at time of placement.

Crushed stone used in bedding and backfilling up to a point twelve inches (12") above the top of the sewer shall be included in the payment for sewer pipe and will not be measured for payment.

Crushed stone used in trench backfill under all roads, areas subject to traffic and other designated areas will **not** be measured for payment. Payment for this crushed stone backfill material will be included in the payment for sewer pipe.

Crushed stone used as base material for pavement replacement also will not be measured for payment inasmuch as payment for this material will be included in the payment for pavement replacement.

Crushed stone used as surface replacement in stone driveways, street shoulders, etc., shall be measured and paid for by the same method as pavement replacement.

Crushed stone required for maintenance of unpaved drives, roads, shoulders shall be at the **CONTRACTOR'S** expense and will not be measured for payment.
b. **Payment** - Payment for crushed stone, measured as provided above, will be made on the basis of the unit price bid per ton, which payment shall constitute full compensation for furnishing, hauling, placing and compacting the stone as specified.

10.11 **EXTRA DEPTH TRENCH EXCAVATION**

a. **Measurement** - Measurement of extra depth trench excavation required to remove mucky or unstable material will be based on the linear feet of excavation involved multiplied by the maximum allowable trench width (O.D. + 18") and the actual depth of such extra excavation as ordered by the ENGINEER giving a volume to the nearest 0.1 C.Y.

b. **Payment** - Payment for extra depth trench excavation will be made on the basis of the unit price bid per cubic yard and shall constitute full compensation for all associated items including disposal of unstable material and incompressible material required to bring trench back to grade. Crushed stone and pavement replacement are covered in Sections 11.10 and 11.09 respectively.

10.12 **OTHER WORK**

The method of payment for the project shall be as described in the preceding items of this section and as set out in the Bid Form. Any other items of work necessary to complete the project in accordance with the plans and specifications shall be included in the prices bid for the herein listed pay items and no separate payment will be made for such work.
STANDARD DETAILS

FIGURE 1

EXTEND GEOTILE BEYOND DOWNSTREAM TOE OF DAM

"L"

"A"

"B"

5'

FIGURE 2

GEOTEXTILE

FLOW

A

2:1

24" MAX.

NOTES:

1. SEE PLAN FOR STONE CHECK DAM OR ROCK CHECK DAM INSTALLATION

2. L = THE DISTANCE SUCH THAT POINTS "A" AND "B" ARE OF EQUAL ELEVATION.

SECTION "A-A"

ROCK CHECK DAM DETAIL

EC-1

N.T.S.
NOTES:
1. FILTER FABRIC FENCE TO BE PLACED PRIOR TO START OF ROUGH GRADING.
2. STEEL POSTS SHALL BE APPROVED BY OWNER PRIOR TO USE.
3. WOOD POSTS SHALL BE 2" x 2" MIN., OAK OR SIMILAR HARDWOOD.
4. POSTS SHALL BE SPACED AT 6' INTERVALS.
5. WIRE FABRIC & FILTER FABRIC SHALL BE SECURELY BOUND TO POSTS WITH EITHER STAPLES OR WIRE TIES.
6. FILTER FABRIC SHALL BE POLYPROPYLENE FABRIC WITH EQUIVALENT OPENING SIZE (EOS) OF NO.100 SIEVE MIN., NO.40 SIEVE MAX., AS DETERMINED BY CORPS OF ENGINEERS GUIDE SPEC. CW 92215.

SILT FENCE DETAIL

EC2
BRACE RAIL AT EACH GATE, CORNER, PULL AND END POST.

3/8" DIAMETER ADJUSTABLE ROD AT CORNER, END, GATE, AND PULL POSTS.

END, CORNER AND PULL POST 2-7/8" D.D. PIPE - 5.75 LBS PER FOOT.

TYPICAL FENCE & GATE DETAIL (1 OF 3)

N.T.S.
NOTES:

1. PROVIDE POSITIVE TYPE LATCHING DEVICE WITH PROVISIONS FOR PADLOCKING, CENTER PLUNGER ROD, CATCH AND SEMI-AUTOMATIC OUTER CATCHES.

2. BARBED WIRE TO BE THREE 12-1/2 GAUGE STRANDS WITH 14 GAUGE 4 POINT BARBS AT 5" CENTERS AND COATED WITH 0.25 OSF ZINC COATING.

3. FENCE FABRIC TO BE 9 GAUGE STEEL, 2.0 OSF ZINC COATED AND WOVEN INTO 2" DIAMOND MESH.

4. FABRIC CONNECTIONS AS FOLLOWS: TERMINAL POSTS - 3/16" x 3/4" STRETCHER BAR WITH 1/8" x 7/8" STEEL BANDS @ 16" O.C.; LINE POSTS - 9 GAUGE WIRE CLIPS @ 15" O.C.; TOP RAIL - 9 GAUGE TIE WIRE @ 24" O.C.

5. ALL POSTS AND OTHER APPURTENANCES SHALL BE HOT DIPPED GALVANIZED WITH MINIMUM 1.2 OSF ZINC COATING. ALL FITTINGS SHALL BE MALLEABLE DUCTILE IRON OR STEEL.

TYPICAL FENCE & GATE DETAIL (3 OF 3)
FORCE MAIN AIR RELEASE / AIR VACUUM VALVE DETAIL

N.T.S.
CONCRETE CRADLE ON RIGID NON-REINFORCED PIPE VARES
SLOPE TO CHANNEL 2 1/2 : 12
SLOPE CHANNEL FROM INLET(S) TO OUTLET A MINIMUM OF 0.2 FEET ACROSS MANHOLE AND NO GREATER THAN 0.5 FEET.
FORMED INVERT-PLAN

SHALLOW MANHOLE

PRECAST TOP-PLAN

STANDARD PRECAST MANHOLE DETAIL (1 OF 2)
N.T.S.

GS-1
NOTES:

1. SEE PLAN FOR LOCATION, ELEVATION AND PIPE INFORMATION.
2. FLOWLINE OF MANHOLE MAY BE FORMED IN CONCRETE, BUILT UP WITH MORTAR, OR BY LAYING A PIPE THROUGH MANHOLE. THE TOP BEING BROKEN OUT LATER.
3. GROUT ANNUAL SPACE BETWEEN WALLS AND PIPE WITH NON-SETTLE MORTAR TO INSURE WATER-TIGHT SEAL.
4. ALL CONCRETE TO COMPLY WITH ACI 318 WITH A M.F. STRENGTH OF 4,000 PSI AT 28 DAY TEST.
5. ALL PRECAST RISERS AND TOPS TO CONFORM TO ASTM C49S.
6. MANHOLE TO BE CONSTRUCTED OF SUFFICIENT SIZE TO ACCOMMODATE INLET AND OUTLET PIPES.
7. MANHOLE STEPS TO BE M.A. INDUSTRIES, PROVIDED WITHIN 12" OF TOP, 12" O.C.
8. SANITARY MANHOLE COVERS TO HAVE "SANITARY COVER" CAST ON THE COVER AND BE JOHN BOUCHARD & SONS NO. 1150. STORM MANHOLE COVERS TO HAVE "STORM" CAST ON THE COVER AND BE JOHN BOUCHARD & SONS NO. 1155.
9. GRADE RINGS ARE REQUIRED TO ADJUST TOP ELEVATION PRIOR TO FINAL PAVING.
10. KOR-N-SEAL BOLT CONNECTORS OR APPROVED EQUAL REQUIRED.
NOTE:
WHERE GASKET MATERIAL DOES NOT PROTRUDE FROM JOINTS, 
FILL UP JOINT WITH GROUT, WHETHER INSIDE OR OUTSIDE.

PLASTIC GASKET JOINT FOR PRECAST MANHOLE DETAIL

N.T.S.

GS-2
NOTE: MANHOLE STEPS TO BE 3/8" STEEL REINFORCED ROD ENCAPSULATED IN POLYPROPYLENE PLASTIC OR EQUAL.

TYPICAL MANHOLE STEP DETAIL

N.T.S. GS-2.1
NOTES:

1. MANHOLE STEPS SHALL BE M.A. INDUSTRIES, INC. CR APPROVED ECOUL.

2. STEP SHALL BE STEEL REINFORCED AND SHALL BE ENCAPSULATED IN POLYPROPYLENE PLASTIC.

3. 1/2" DIAMETER STEEL REINFORCEMENT (GRADE 60).

SECTION "A-A"
NOTE:
1. WHERE "N" IS EQUAL TO 8"-18", "M" WILL BE EQUAL TO 8".
2. WHERE "N" IS EQUAL TO 21"-30", "M" WILL BE EQUAL TO 12".
3. THE CENTERLINE OF THE DROP SEWER LINE SHALL BE AT THE SAME ELEVATION AS THE SPRING LINE OF THE LARGEST SEWER OR PROVIDE A MINIMUM OF 0.2 FEET DROP ACROSS THE MANHOLE.

DROP MANHOLE DETAIL
N.T.S.  GS-2.4
1. VENT TO BE LOCATED BEHIND CURB OR WALK AND FAR FROM ROADWAY SO AS TO PREVENT TRAFFIC FLOW OBSTRUCTION, OR AS NOTED ON THE PLANS.

2. ALL VENT PIPING IS TO BE PRIMED WITH ONE COAT OF RED PRIMER AND TWO COATS OF GREEN ENAMEL.

3. TOP OF VENT TO BE A MINIMUM OF 8'-0" ABOVE GRADE OR AS NOTED ON THE PLANS.

STANDARD MANHOLE VENT

N.T.S.

GS-2.5
NOTES:
1. TAPERED CONCRETE COLLARS ARE REQUIRED IN BOTH PAVED AND NON-PAVED AREAS.
2. SERVICE LINES ARE TO BE BACKFILLED WITH CRUSHED STONE TO A POINT APPROXIMATELY
   2' BEYOND THE ROAD SHOULDER OR CURB IF LOCATED IN A PAVED AREA.
3. CLEANOUTS ARE TO BE LOCATED AT THE PROPERTY LINE. ADDITIONAL CLEANOUTS REQUIRED ON
   SERVICE LINES SHALL BE INSTALLED NO FARTHER THAN 15 FEET APART.
4. CLEANOUTS SHALL BE 6" SCH 40 PVC FROM THE SEWER MAIN TO THE CLEANOUT.
5. THE CLEANOUT CAP SHALL BE A JONES STEPHENS CO. 6" GASKETED CAST IRON PUSH-ON CLEANOUT WITH A
   BRONZE PLUG OR AN APPROVED EQUAL. THE PLUGS SHALL HAVE A COUNTERSUNK HEAD IN PAVED AREAS.
6. CLEANOUTS LOCATED WITHIN A ROADWAY, DRIVEWAY, OR PARKING LOT SHALL BE PROTECTED FROM TRAFFIC
   USING A CAST IRON BOX AS SHOWN IN THE "SEWER CLEANOUT BOX DETAIL" (Fig-61).

SANITARY SEWER SERVICE LINE CONNECTION DETAILS (1 OF 2)
NOTES:
1. TAPERED CONCRETE COLLARS ARE REQUIRED IN BOTH PAVED AND NON-PAVED AREAS.
2. SERVICE LINES ARE TO BE BACKFILLED WITH CRUSHED STONE TO A POINT APPROXIMATELY 2' BEYOND THE ROAD SHOULDER OR CURB IF LOCATED IN A PAVED AREA.
3. CLEAONUTS ARE TO BE LOCATED AT THE PROPERTY LINE. ADDITIONAL CLEAONUTS REQUIRED ON SERVICE LINES SHALL BE INSTALLED NO FARTHER THAN 15 FEET APART.
4. CLEAONUTS SHALL BE 6" SDW 35 PVC FROM THE SEWER MAIN TO THE CLEAONUT.
5. THE CLEAONUT CAP SHALL BE A JONES STEPHENS CO. 6" GATED CAST IRON PUSH-ON CLEAONUT WITH A BRONZE PLUG OR AN APPROVED EQUAL. THE PLUGS SHALL HAVE A COUNTERSINK HEAD IN PAVED AREAS.
6. CLEAONUTS LOCATED WITHIN A ROADWAY, DRIVEWAY, OR PARKING LOT SHALL BE PROTECTED FROM TRAFFIC USING A CAST IRON BOX AS SHOWN IN THE "SEWER CLEAONUT BOX DETAIL" (25-4).

**TEER BRANCH (SEWER DEPTH GREATER THAN 7'-0"
SANITARY SEWER SERVICE LINE CONNECTION DETAILS (2 OF 2)**

N.T.S.
NOTE:
EDGE OF PAVEMENT TO BE TRIMMED A MINIMUM OF 12" BEYOND EACH SIDE OF TRENCH WIDTH TO OBTAIN NEAT LINES. COLD MIX TO BE PLACED AS A TEMPORARY SURFACE WITHIN 48 HOURS OF MAKING ROAD CROSSING.
BACKFILL IN PAVED AREAS SHALL BE CRUSHED STONE. SEE SPECIFICATIONS.

1. DETAILS ARE BOTH APPLICABLE TO BOTH EARTH AND ROCK TRENCHES.

2. COMPACTED CRUSHED STONE SHALL BE SIZE 57 AS GIVEN IN SECTION 903 OF THE T.O.Q.T. STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION.

STANDARD TRENCH DETAIL
N.T.S.  ST-1
STANDARD CONNECTION OF FORCE MAIN TO MANHOLE

N.T.S
HIGH-DENSITY POLYETHYLENE CONSTRUCTION METER BOX (CARSON INDUSTRIES NO. MSBCF 1720-18X) WITH NO. 1730 SOLID COVER CONSTRUCTED OF POLYMER CONCRETE

1-1/2" or 2" BADGER METER
GRADE
CAP OR PLUG

6" DIA. PVC VALVE BOX
1-1/2" or 2" PRESSURE REDUCING VALVE (WILKINS SERIES 600 OR EQUAL)

1 1/2"-2" CURB STOP
TYPE "L" COPPER

WATER MAIN
BRICK SUPPORT
CORPORATION COCK (MUELLER H-15000 SERIES OR EQUAL)
TAPPING SADDLE (MUELLER 13400 SERIES OR EQUAL)
GRAVEL

SCH 40 PVC
TO CUSTOMER

METER YOKE w/ CHECK VALVE, ANGLE STOP AND PROVISION FOR LOCKING w/ REQUIRED SERVICE

1-1/2" OR 2" SERVICE ASSEMBLY w/ PRESSURE REDUCING VALVE
N.T.S.
APPENDIX A

Town of Smyrna
As-Built Check List

On Plans:

1. Contact information
2. Subdivision name and section number
3. Plat Book and Page numbers
4. Lot numbers
5. (2) Dimensions to the following:
   a. Water meters
   b. Water valves
   c. Fire hydrants
   d. Storm drain features
   e. Sanitary clean outs
   f. Above ground electrical or cable boxes

6. Stationing to the following:
   a. Manholes
   b. Sanitary tap locations from downstream manhole
   c. Offset length of sewer service
   d. Depth at end of sewer service

   Items b, c, and d should be as illustrated in figure 1.1

7. Elevations to the following:
   a. Manholes
      1) Top-of-cast
      2) Invert to bottom, center
   b. Storm drain inlets top, center
   c. Plan and Profile are required for sanitary sewer, retention and detention areas

In Field:

1. Blue paint on water valve and at 90 degrees to the valve on the curb.

Dimensions should be kept as close to or under 100’ as possible. Dimension ties must be made to a semi-immovable item. Examples of acceptable semi-immovable features would be; manholes, power (light) poles, storm drain headwalls. Where the determination is made that there is not an acceptable feature to dimension to, drill holes can be made and painted in the top-of-curb and measured to the feature.